

SERVICING AND STORMWATER MANAGEMENT REPORT - 355 FRANKTOWN ROAD



Project No.: CCO-22-0402

Prepared for:

11309455 Canada Inc
190 Lisgar St,
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Prepared by:

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McINTOSH PERRY

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1.0 PROJECT DESCRIPTION

1.1 Purpose

McIntosh Perry (MP) has been retained by the 11309455 Canada Inc to prepare this Servicing and Stormwater Management Report in support of the site plan approval for the proposed development at 355 Franktown Road within the Town of Carleton Place.

The main purpose of this report is to demonstrate that the proposed development has access to sufficient public services in accordance with the recommendations and guidelines provided by the Town of Carleton Place (Town), the Mississippi Valley Conservation Authority (MVCA) and the Ministry of the Environment, Conservation and Parks (MECP). This report will address access to water, sanitary and storm servicing for the development, ensuring that existing services will adequately service the proposed development.

1.2 Site Description

The property is located at 355 Franktown Road in the Town of Carleton Place. The site, which is not considered to include the commercial plaza, covers approximately 1.34 ha and is located between the proposed second phase of Coleman Street Subdivision and Franktown Road.

The existing site is currently undeveloped, consisting of wooded and grassed areas. Adjacent lots to the north and south are also undeveloped. Coleman Street Subdivision Phase 2 flanks the eastern portion of the property and existing commercial and residential developments along Franktown Road are located to the west.

The development proposes two 4-Storey condominium buildings on the western portion of the property and six townhouses on the eastern portion of the property. The condominium buildings will be separated from the townhouse blocks by a public ROW. The future ROW will connect the proposed development to the lands to the north and eventually to the Coleman subdivision via the lands to the south.

2.0 BACKGROUND DOCUMENTS

Background documents available under separate cover include:

- JLR Watermain Capacity – Future Development_Final (Dated September 16, 2013 completed by J.L. Richards & Associates Ltd.)
- Functional Servicing Report – 347 Franktown Road (Dated August 13, 2021 completed by McIntosh Perry Consulting Engineers Ltd.)
- Servicing and Stormwater Management Report – 347 Franktown Road (Date June 22, 2022 completed by McIntosh Perry Consulting Engineers Ltd.)
- Servicing and Stormwater Management Report – Coleman Central Subdivision Phase 2 (Dated February 12, 2024) Note: This subdivision is currently ongoing approvals. Servicing and references will be updated to reflect the approved documents when complete.

3.0 WATERMAIN

3.1 Existing Watermain

The following subsections outline the existing water infrastructure within Franktown Road and the proposed infrastructure within Coleman Street Subdivision Phase 2.

3.1.1 Franktown Road

There is an existing 200 mm diameter watermain that runs north along Franktown Road, ending in a stub located at Findlay Avenue. Just before the stub there is a hydrant that services the existing commercial development adjacent to the subject site.

3.1.2 Coleman Central Subdivision

Although not yet constructed, the infrastructure within the proposed Coleman Central Subdivision Phase 2 is anticipated to be constructed prior to the proposed construction of the subject property. There is a proposed 200 mm diameter watermain that services the subdivision. The design of the Coleman Street Subdivision Phase 2 has taken the future development into account with stubs extending westward from the subdivision located both northeast and southeast of the subject site.

3.2 Proposed Watermain

The existing 200 mm watermain within Coleman Central Subdivision Phase 2 will be extended along the future municipal road. In accordance with the Watermain Capacity – Future Development provided by the Town of Carleton Place, a new 200mm watermain is proposed to connect the extended main within the future municipal ROW. A 150mm PVC water lateral will extend from the proposed 200mm watermain to service the condo buildings, as shown on drawing C102. The townhouse block will be serviced via 19mm copper 'k' type laterals extending from the 200mm watermain within the future municipal road. A new service will be extended to the existing mall from the proposed 200mm watermain within the site. The proposed watermain will be extended through the site and connect to the existing municipal watermain within Franktown Road.

The Fire Underwriters Survey 2020 (FUS) method was utilized to determine the required fire flow for the proposed Phase 1 development. All buildings in the development were evaluated for the worst-case fire flow scenario. It was determined that the townhouse block is the worst case. Detailed water and fire calculations for the development can be found in Appendix 'C' of this report.

The 'C' factor (type of construction) for the townhouses was determined to be 1.5 (Wood Frame). The total floor area ('A' value) for the FUS calculation was determined to be 1132.0 m². The results of the calculations yielded a required fire flow of 11,000 L/min. The detailed calculations for the FUS can be found in Appendix 'C'.

The water demands have been calculated to adhere to the *Ottawa Design Guidelines – Water Distribution* manual and can be found in Appendix 'C'. *Table 1*, below, summarizes the design criteria and total calculated demands.

Table 1: Water Supply Design Criteria and Water Demands

Water Demand Rate (Commercial)	28,000 L/gross ha/d
Water Demand Rate (Residential)	280 L/c/day
1-Bedroom Apartment	1.4 Persons/unit
2-Bedroom Apartment	2.1 Persons/unit
Townhouse	2.7 Persons/unit
Residential Peaking Factor (Day)	4.9 x avg. day
Residential Peaking Factor (Hour)	7.4 x max. day
Site Area (ha)	2.07
Average Day Demand (L/s)	0.86
Maximum Daily Demand (L/s)	3.42
Peak Hourly Demand (L/s)	5.27
FUS Fire Flow Requirement (L/s)	183.33
Max Day + Fire Flow (L/s)	186.75

3.3 Hydraulic Water Model Results

With reference to the Watermain Capacity – Future Development Pg. 18, pressures under peak demand were analyzed and a hydraulic water model was completed using Bentley's WaterCAD modelling software based on those conditions. A total of three (3) scenarios were analyzed. The performance of the proposed water distribution system within the development was analyzed under each scenario. The following summarizes the modelling scenarios that were analyzed.

- Scenario 1: Average Day Demands (w/ Maximum HGL)
- Scenario 2: Peak Hour Demands (w/ Minimum HGL)
- Scenario 3: Max Day Plus Fire Flow (w/ Reduced Minimum HGL)

The normal operating pressure range is anticipated to be 449 kPa to 462 kPa and will not be less than 275 kPa (40 psi) or exceed 689 kPa (100 psi). *Table 2*, below, summarizes the resultant water pressures at each junction per scenario.

Table 2: Water Pressure at Junctions per Scenario

Junction	Scenario 1: Average Day Demand (psi)	Scenario 2: Peak Hourly Demand (psi)
J-2	67	67
J-3	68	68
J-4	68	67
J-5	68	68
J-6	68	67
J-7	68	67
J-8	68	68
J-9	68	68
J-10	68	68
J-11	68	68
J-12	68	68
J-13	72	71
J-14	68	67
J-15	68	67
J-16	67	66
J-17	66	65
J-18	64	63
J-19	64	63

To analyze the maximum day demands plus fire flow scenario, the fire flow calculation tool in the water modelling software was used to run multiple iterations of the scenario while gradually increasing fire flows being applied to a single junction until the minimum pressure of 20 psi is reached at any point in the system. A summary of the maximum available fire flow results is provided in Appendix C. Please note the results are considered conservative, as reductions were applied to the HGL at the connection point within Franktown Road.

The water model results determined that the proposed 200mm watermain can adequately provide enough fire flow to meet the required flow rate of 11,000 L/min (183.33 L/sec) at the location of the proposed hydrants H-4 and H-3 (junctions J-15 and J-19), with available fire flows ranging from 13,532 L/min to 11,488 L/min (225.54 L/sec to 191.46 L/sec) while maintaining a minimum residual pressure of 20 psi in the network. Fire flow to the

proposed townhouse block will be provided in part by the proposed hydrant within the Coleman Subdivision, given the distance between Hydrant H-4 and the townhouse block will exceed 150m. The water model results determined that the proposed hydrant within the Coleman Subdivision (J10) will provide 15,312 L/min of fire flow, or 255.20 L/s, while maintaining a residual pressure of 20 psi in the network. Refer to the Hydraulic Water Modelling results and figure C1 in Appendix C for more details.

To provide fire flow to the proposed condo buildings internal fire suppression system, a private hydrant (H-4) within 45m of the siamese connection is proposed. A hydrant summary based on the water model can be seen in *Table 3*, below.

Table 3: Fire Protection Confirmation

Building	Max Fire Flow Demand (L/min.)	Fire Hydrant H-3 (L/min.)	Fire Hydrant H-4 (L/min.)	Coleman Subdivision Hydrant (J10) (L/min.)	Combined Fire Flow (L/min.)
Condo Buildings	9,000	13,532.4	11,488	-	>9,000
Townhouse Block	11,000	13,532.4	-	15,312	>11,000

4.0 SANITARY DESIGN

4.1 Existing Sanitary Sewer

Although not yet constructed, Coleman Street Subdivision Phase 2 has a proposed 200 mm diameter sanitary sewer with stubs located to the northeast and southeast of the subject site.

4.2 Proposed Sanitary Sewer

The 200 mm sanitary sewer stub within Coleman Street Subdivision is proposed to be extended along the future municipal road to service the subject property. A 200 mm sanitary sewer is proposed to be extended from the municipal road within the drive aisles bounding the condo buildings. The condo buildings will have shared servicing through a 200 mm sanitary service connection to the proposed 200 mm diameter sanitary sewer. The proposed sewer will also service the existing mall to the west. Each townhouse will be serviced by 135mm sanitary laterals extending from the 200mm sewer within the future municipal road. Refer to drawing C102.

The peak design flow was calculated for the proposed site using the Ottawa Sewer Design Guidelines (SDG). Design criteria used in the sanitary demand calculation can be seen in *Table 4*, below.

Table 4: Sanitary Design Criteria

1-Bedroom Apartment	1.4 persons/unit
2-Bedroom Apartment	2.1 persons/unit

Townhouse	2.7 persons/unit
Average Daily Demand	280 L/day/person
Site Area (Condos, Townhouses, and Existing Mall))	2.07 ha
Residential Peaking Factor	3.52
Commercial	2,800 L/(1000m ² /d)
Extraneous Flow Allowance	0.33 L/s/ha

Table 5, below, summarizes the estimated wastewater flow from the proposed development. Wastewater flows from the proposed Chadha development are not included in this summary but have been accounted for in sanitary sizing and capacity. Detailed calculations for each contributing area can be found in Appendix 'D'.

Table 5: Summary of Estimated Sanitary Flow

Average Dry Weather Flow	0.97L/s
Peak Dry Weather Flow	2.66 L/s
Peak Wet Weather Flow	3.24 L/s

Based on the calculation provided in the Coleman Central Subdivision Phase 2 Servicing Report and the results shown in Table 5, above, it is anticipated that there will be no downstream capacity concerns. Flow from the subject site has been accounted for in the Coleman Central Subdivision design, refer to subdivision design documents for details.

Further to the above, the town has initiated its own analysis to confirm the capacity of the receiving network.

5.0 STORM DESIGN

5.1 Existing Storm Sewer

There is no existing storm infrastructure within the subject property. Stormwater runoff currently sheet drains to the southeast where it is collected by the existing creek. The existing mall adjacent to the site currently outlets to a storm water management area within the development. There is a 975mm concrete storm sewer to be extended from the Coleman Phase 2 subdivision specifically to provide an outlet for 347 and 355 Franktown developments. The 975mm sewer ultimately outlets to an existing ditch that has been realigned as part of the Coleman Central Subdivision Phase 2 development. Please refer to the subdivision documents for details.

5.2 Proposed Storm Sewer

The proposed development will be serviced by a new storm network extended from the future 975mm storm sewer within the future municipal road that will be extended from the existing storm sewer within Coleman Central Subdivision Phase 2. A new outlet to the realigned ditch within the Coleman Central Subdivision Phase 2 pond block is proposed to accommodate flows from the proposed development. As part of the ditch realignment, flows from the subject site have been considered. As existing flows from the adjacent mall

currently flow to the site, they will also be considered in the proposed storm water management network and restricted.

Runoff from the condo buildings, drive aisle, rear yard, existing mall and southern landscaped area will be captured and restricted.

Flow attenuation for the above-mentioned areas will be provided via a 180mm plug style orifice located on the upstream invert of the outlet pipe for the ponding area. Flows greater than the allowable release rate will be stored in a landscape area complete with a 2.00m weir at the southeast of the site.

Runoff from the townhouses and the proposed municipal road will sheet drain without attenuation to the future municipal Row.

6.0 STORMWATER MANAGEMENT

6.1 Design Criteria and Methodology

Stormwater management for the proposed site will be maintained through attenuated surface storage provided in a landscape area the southeast of the site. Catch basins will be collect runoff from at-grade areas within the site. The quantitative and qualitative properties of the storm runoff for both the pre & post development flows are further detailed below. The post-development 5 and 100-year flows will be restricted to the pre-development 5 and 100-year flows.

6.2 Runoff Calculations

Runoff calculations presented in this report are derived using the Rational Method, given as:

$$Q = 2.78CIA \text{ (L/s)}$$

- Where
- C = Runoff coefficient
 - I = Rainfall intensity in mm/hr (City of Ottawa IDF curves)
 - A = Drainage area in hectares

It is recognized that the Rational Method tends to overestimate runoff rates. As a result, the conservative calculation of runoff ensures that any stormwater management facility sized using this method is anticipated to function as intended.

The following coefficients were used to develop an average C for each area:

Roofs/Concrete/Asphalt	0.90
Gravel	0.60
Undeveloped and Grass	0.20

As per the *City of Ottawa - Sewer Design Guidelines*, the 5-year balanced 'C' value must be increased by 25% for a 100-year storm event to a maximum of 1.0.

The time of concentration (Tc) used for pre-development and post-development shall be calculated using a Tc of 10 minutes.

6.3 Pre-Development Drainage

The existing site drainage limits are demonstrated on the Pre-Development Drainage Area Plan. A summary of the Pre-Development Runoff Calculations can be found in *Table 6, below*. Please note the SWM area and Site Area vary slightly as a portion of the townhouse block will be directed to Coleman Phase 2 and accounted for in their stormwater management calculations.

Table 6: Pre- Development Runoff Summary

Drainage Area	Area (ha)	Runoff Coefficient (5-Year)	Runoff Coefficient (100-Year)	5-year Peak Flow (L/s)	100-year Peak Flow (L/s)
A1	1.33	0.20	0.25	77.11	165.18
A2	0.69	0.20	0.25	40.04	85.78
A3	4.47	0.20	0.25	259.20	555.25

See CCO-22-0402 - *PRE* in Appendix 'E' and Appendix 'G' for calculations.

6.4 Post-Development Drainage

The proposed site drainage limits are demonstrated on the Post-Development Drainage Area Plan. See CCO-22-0402 - *POST* in Appendix 'F' of this report for more details. A summary of the Post-Development Runoff Calculations can be found in *Table 7, below*.

Table 7: Post Development Flow Rate

Drainage Area	Area (ha)	Runoff Coefficient (5-Year)	Runoff Coefficient (100-Year)	5-year Peak Flow (L/s)	100-year Peak Flow (L/s)
B1	0.28	0.47	0.54	37.77	74.32
B2	0.74	0.64	0.73	137.34	265.21
B3	0.57	0.87	0.97	143.13	272.82
B4	0.32	0.56	0.64	51.37	99.91
B5	4.47	0.20	0.25	259.20	555.25
Total	6.36			628.80	1267.51

See Appendix 'G' for calculations. Runoff for area B1-B3 will be restricted before draining to the sewer within the future municipal ROW. The flow will be controlled through the use of a 180mm plug style ICD. Runoff from areas B4 will leave the site unrestricted. Quantity and quality control will be further detailed in Sections 6.5 and 6.7.

6.5 Quantity Control

The total post-development runoff for this site has been restricted to match the 5-year and 100-year pre-development flow rates calculated with a combined C value. Note that areas A3 and B5 are offsite and will outlet to the storm sewer within the future public road at full buildout conditions therefore these areas are not included in the site quantity calculations. These values create the following allowable release rate and storage volumes for the development site.

Table 8: Allowable Release Rate Summary

Drainage Area	Area (ha)	Runoff Coefficient 5-Year	Runoff Coefficient 100-Year	Required Restricted Flow 5-Year (L/s)	Required Restricted Flow 100-Year (L/s)
A1	1.33	0.20	0.25	77.11	165.18
A2	0.69	0.20	0.25	40.04	85.78
Total	2.02			117.15	250.95

See Appendix 'G' for calculations.

Reducing site flows will be achieved using a flow restriction and will create the need for onsite storage. Runoff from area B1 to B3 will be restricted as shown in *Table 9*, below.

Table 9: Post-Development Restricted Runoff Summary

Drainage Area	Post Development Unrestricted Flow (L/s)		Post Development Restricted Flow (L/s)		
	5-Year	100-Year	5-Year	100-Year	
B1	37.77	74.32	55.77	135.10	Restricted – ICD
B2	137.34	265.21			
B3	143.13	272.82			
B4	51.37	99.91	51.37	99.91	Unrestricted
Total	369.60	712.25	107.13	235.01	

See Appendix 'G' for calculations.

Runoff from areas B1 to B3 will be restricted using an ICD within the inlet of DICB5. This will backup stormwater runoff from the site to a landscaped area southeast of the site. The area will pond to elevations of 133.17 and 133.47 for the 5-year and 100-year storms, respectively. The landscaped area will be complete with a 2.00m earth weir.

A storage summary can be seen in *Table 10*, below.

Table 10: Storage Summary

Drainage Area	Storage Required (m ³)	Storage Available (m ³)	Storage Required (m ³)	Storage Available (m ³)
	5-Year		100-Year	
B1	195.9	199.9	331.7	353.7
B2				
B3				

6.6 Hydraulic Grade Line Analysis

The hydraulic grade line was reviewed within the proposed storm sewer network to evaluate the need for sump pumps within the proposed condo buildings and townhouse block. PCSWMM was used to evaluate the HGL based on a 100-year Chicago Storm with a 3-hour duration. The results of the HGL analysis indicated that sump pumps will be required for the townhouse block, as the 100-year HGL elevation will be greater than the USF elevation. Results can be found in *Table 11*, below. Please refer to Appendix 'G' for additional information.

Table 11: Junction HGL vs USF Elevation

Max HGL (m) (MH111)	Max. HGL (m) (MH106)	USF Elev. (m) (Condo Buildings)	USF Elev. (m) (Townhouse Block)
131.85	131.70	132.75	131.36

Additional notes have been added to drawing C102 regarding the requirement for sump pumps and back-flow preventors.

6.7 Quality Control

The development of this lot will employ Best Management Practices (BMP's) wherever possible. The intent of implementing stormwater BMP's is to ensure that water quality and quantity concerns are addressed at all stages of development. BMP's at this site will be implemented at the lot level. Lot level BMP's typically include temporary retention of the parking lot runoff, minimizing ground slopes and maximizing landscaped areas.

A quality treatment unit has been sized to provide a TSS removal rate of 80% as per the Mississippi Valley Conservation Authority (MVCA) requirements. The Oil and Grit Separator (OGS) will provide a water quality of at least 80% TSS. The OGS Unit shall be placed downstream of the restriction unit to provide the required water quality treatment for the site runoff before discharging to the existing creek southeast of the site.

7.0 EROSION AND SEDIMENT CONTROL

7.1 Temporary Measures

Before construction begins, temporary silt fence, straw bale or rock flow check dams will be installed at all natural runoff outlets from the property. It is crucial that these controls be maintained throughout construction

and inspection of sediment and erosion control will be facilitated by the Contractor or Contract Administration staff throughout the construction period.

Silt fences will be installed where shown on the final engineering plans, specifically along the downstream property limits. The Contractor, at their discretion or at the instruction of the City, Conservation Authority or the Contract Administrator shall increase the quantity of sediment and erosion controls on-site to ensure that the site is operating as intended and no additional sediment finds its way off site. The rock flow, straw bale & silt fence check dams and barriers shall be inspected weekly and after rainfall events. Care shall be taken to properly remove sediment from the fences and check dams as required. Fibre roll barriers are to be installed at all existing curb inlet catchbasins and filter fabric is to be placed under the grates of all existing catchbasins and manholes along the frontage of the site and any new structures immediately upon installation. The measures for the existing/proposed structures are to be removed only after all areas have been paved. Care shall be taken at the removal stage to ensure that any silt that has accumulated is properly handled and disposed of. Removal of silt fences without prior removal of the sediments shall not be permitted.

Although not anticipated, work through winter months shall be closely monitored for erosion along sloped areas. Should erosion be noted, the Contractor shall be alerted and shall take all necessary steps to rectify the situation. Should the Contractor's efforts fail at remediating the eroded areas, the Contractor shall contact the City and/or Conservation Authority to review the site conditions and determine the appropriate course of action. As the ground begins to thaw, the Contractor shall place silt fencing at all required locations as soon as ground conditions warrant. Please see the *Erosion & Sediment Control Plan* for additional details regarding the temporary measures to be installed and their appropriate OPSD references.

7.2 Permanent Measures

It is expected that the Contractor will promptly ensure that all disturbed areas receive topsoil and seed/sod and that grass be established as soon as possible. Any areas of excess fill shall be removed or levelled as soon as possible and must be located a sufficient distance from any watercourse to ensure that no sediment is washed out into the watercourse. As the vegetation growth within the site provides a key component to the control of sediment for the site, it must be properly maintained once established. Once the construction is complete, it will be up to the landowner to maintain the vegetation and ensure that the vegetation is not overgrown or impeded by foreign objects.

8.0 SUMMARY

- Two new condominium buildings and a block of townhouses are proposed at 355 Franktown Road.
- A new 200mm water main will be extended from the proposed Phase 2 of Coleman Central Subdivision to Franktown Road.
- The FUS method estimated fire flow indicated 11,000 L/min is required for the proposed development.
- Based on boundary conditions provided by the Town, the proposed 200 mm watermain and two private hydrants in the vicinity of the development are capable of meeting daily and fire flow demands.
- A new 200mm sewer main will be installed and connected to the proposed stub at phase 2 of Coleman Central Subdivision

- The development is anticipated to have a peak wet weather flow of 3.24 L/s. A proposed 200 mm diameter sanitary main will collect and outlet flow to the proposed 200 mm diameter sanitary stub located within Phase 2 of the Coleman Central Subdivision. 135mm services will service the block of townhouses, extending from the Phase 2 Coleman sewer. Based on the sanitary analysis conducted in the Coleman Street Subdivision Phase 2 Servicing Report, the subdivisions sanitary network has sufficient capacity for the subject site's flow.
- A new storm system will be installed on-site to capture storm runoff and restrict flows to pre-development rates. The new storm system will discharge future sewer located within Phase 2 of the Coleman Street Subdivision.
- Storage for the 5 and 100-year storm events will be provided via surface storage.

9.0 RECOMMENDATION

Based on the information presented in this report, we recommend that Town of Carleton Place approve this Servicing and Stormwater Management Report in support of the proposed development at 355 Franktown Road.

This report is respectfully being submitted for approval.

Regards,

McIntosh Perry Consulting Engineers Ltd.



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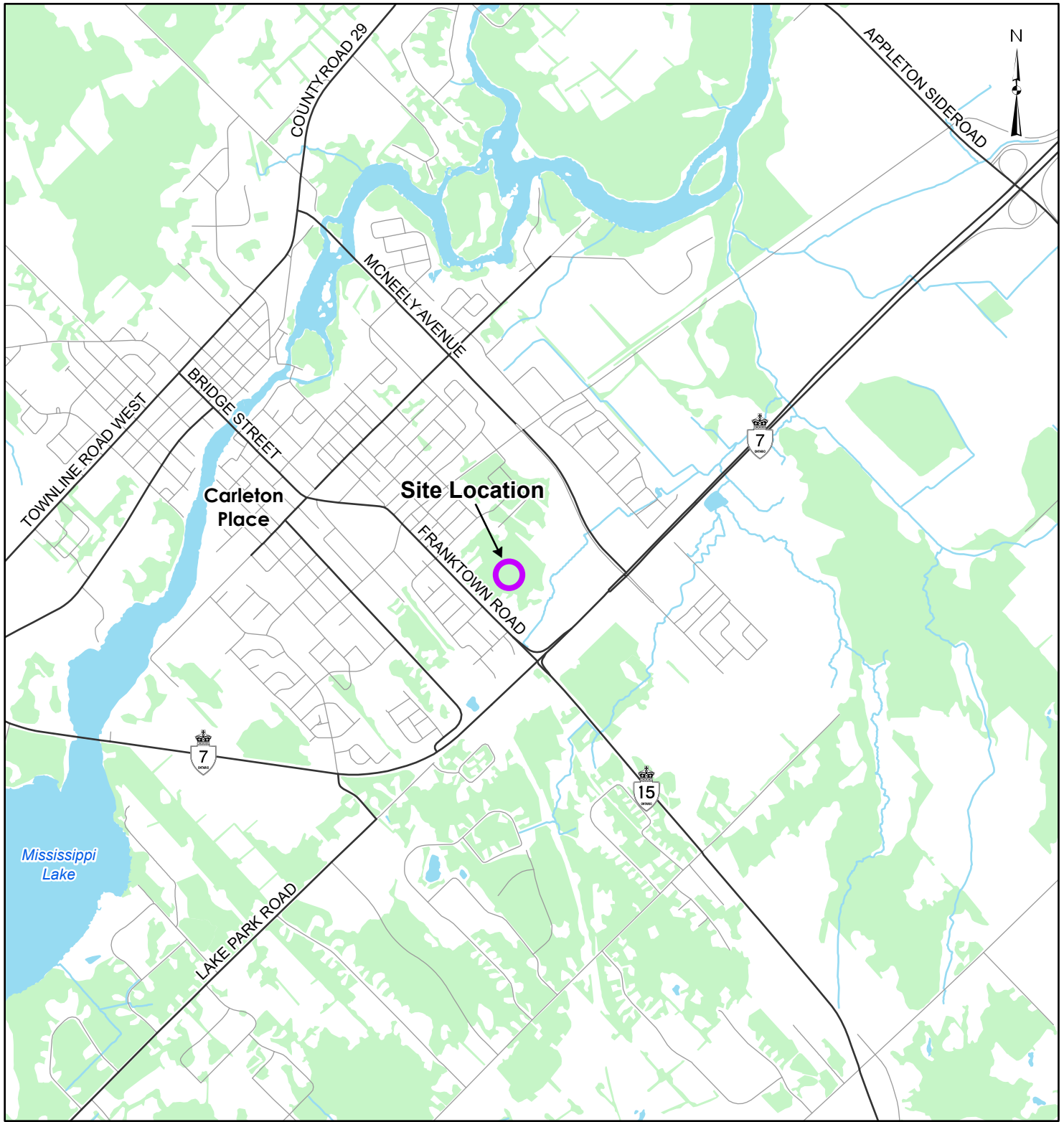
10.0 STATEMENT OF LIMITATIONS

This report was produced for the exclusive use of the 11309455 Canada Inc group. The purpose of the report is to assess the existing stormwater management system and provide recommendations and designs for the post-construction scenario that are in compliance with the guidelines and standards from the Ministry of the Environment, Parks and Climate Change, Town of Carleton Place and local approval agencies. McIntosh Perry reviewed the site information and background documents listed in Section 2.0 of this report. While the previous data was reviewed by McIntosh Perry and site visits were performed, no field verification/measures of any information were conducted.

Any use of this review by a third party, or any reliance on decisions made based on it, without a reliance report is the responsibility of such third parties. McIntosh Perry accepts no responsibility for damages, if any, suffered by any third party as a result of decisions or actions made based on this review.

The findings, conclusions and/or recommendations of this report are only valid as of the date of this report. No assurance is made regarding any changes in conditions subsequent to this date. If additional information is discovered or becomes available at a future date, McIntosh Perry should be requested to re-evaluate the conclusions presented in this report, and provide amendments, if required.

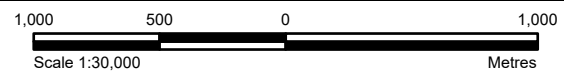
APPENDIX A
KEY PLAN



C:\Users\stunum\Documents\Perry\GIS - Documents\Projects\2018\CCO\CCO-18-0360 Cavanagh, NuGlabe, SubDivision Ph. 2\appx\KeyPlans\CCO-18-0360_01_SiteLocation.aprx

LEGEND

- Site Location
- Local Road
- Major Road
- Watercourse
- Waterbody
- Wooded Area

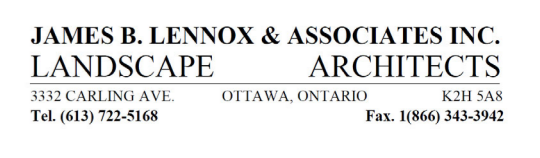
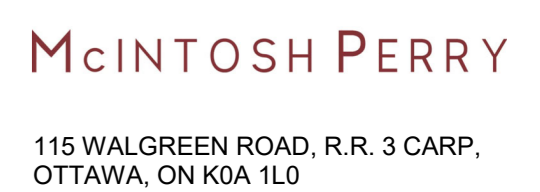


REFERENCE

GIS data provided by the Ontario Ministry of Natural Resources and Forestry, 2021.

CLIENT:	HEAFEY GROUP	
PROJECT:	355 FRANKTOWN ROAD	
TITLE:	SITE LOCATION	
McINTOSH PERRY	PROJECT NO: CCO-22-0402	FIGURE:
Date	Aug., 09, 2021	1
GIS	EU	
Checked By	NV	

APPENDIX B
BACKGROUND DOCUMENTS



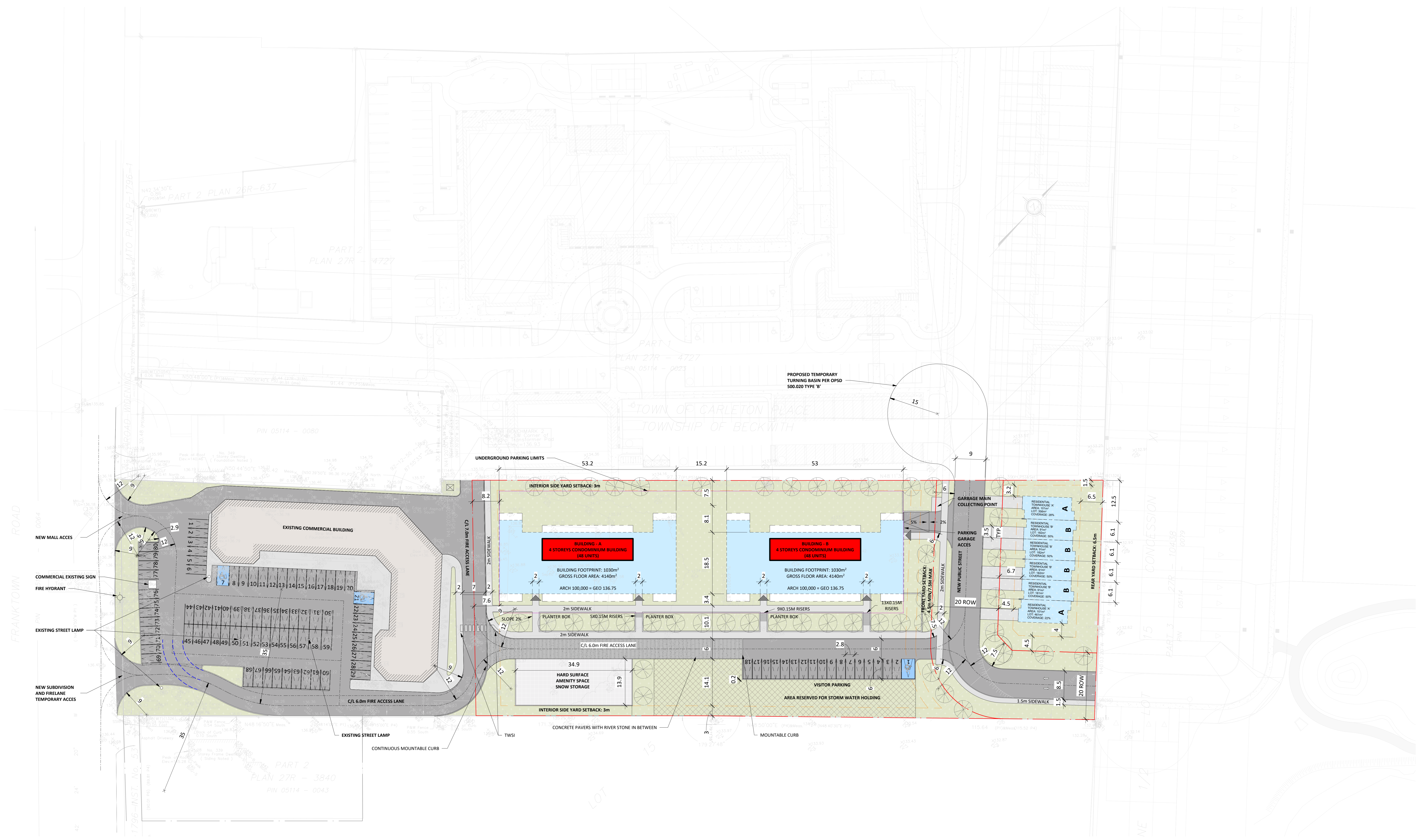
NO	DESCRIPTION	DATE
1	FOR COORDINATION	2024-05-11
2	FOR COORDINATION	2024-05-17
3	FOR COORDINATION	2024-05-17
4	FOR COORDINATION	2024-05-17
5	FOR COORDINATION	2024-05-17
NO	DESCRIPTION	DATE

IT IS THE RESPONSIBILITY OF THE APPROPRIATE CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS ON THE SITE AND TO REPORT ALL ERRORS AND/OR OMISSIONS TO THE ARCHITECT. ALL CONTRACTORS MUST COMPLY WITH ALL PERTINENT CODES AND BY-LAWS. DO NOT SCALE DRAWINGS.

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DO NOT USE FOR CONSTRUCTION

DATE	DESIGNED
2024-05-31	P.POMERLEAU
DATE	DRAWN
	P.POMERLEAU
PROJECT No	CHECKED
21006	P.MARTIN
SHEET TITLE	
SITE PLAN	



C:\Users\pomerleau\OneDrive - PMA Architects\Documents\21006_A_GEO-CONDO_R21_pjpmierleau\W453.rvt

355 FRANKTOWN RD - FRANKTOWN DEVELOPMENT

96 CONDOMINIUMS UNITS
6 FREEHOLD TOWNHOUSES
102 UNITS TOTAL

PARKINGS
RESIDENTIAL COMPLEX
RESIDENTIAL: 116 (RATIO 1.21)
VISITOR: 24 (RATIO .25)
BIKE: 48 (RATIO .5)

COMMERCIAL PLAZA
COMMERCIAL: 80



SITE PLAN LEGEND

- LOT LINE
- BUILDING SETBACK
- SERVITUDE
- 0000 GEO ÉLEVATION GÉODÉSIQUE
- NEW TREE
- DEMOLISHED BUILDING
- EXISTING BUILDING
- LANDSCAPED AREA
- PAVER
- SIDEWALK
- AMPHALT

APPENDIX C
WATERMAIN CALCULATIONS

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Water Demands - BLDG A

Project:	355 Franktown
Project No.:	CCO-22-0402
Designed By:	CH
Checked By:	BSC
Date:	September 1, 2023
Site Area:	0.50 gross ha

Residential	NUMBER OF UNITS		UNIT RATE	
Single Family		homes	3.4	persons/unit
Semi-detached		homes	2.7	persons/unit
Townhouse		homes	2.7	persons/unit
Bachelor Apartment		units	1.4	persons/unit
1 Bedroom Apartment	18	units	1.4	persons/unit
2 Bedroom Apartment	30	units	2.1	persons/unit
3 Bedroom Apartment		units	3.1	persons/unit
Average Apartment		units	1.8	persons/unit
Total Population	89	persons		
Commercial		m2		
Industrial - Light		m2		
Industrial - Heavy		m2		

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	280	L/c/d
Industrial - Light	35,000	L/gross ha/d
Industrial - Heavy	55,000	L/gross ha/d
Shopping Centres	2,500	L/(1000m ² /d)
Hospital	900	L/(bed/day)
Schools	70	L/(Student/d)
Trailer Park with no Hook-Ups	340	L/(space/d)
Trailer Park with Hook-Ups	800	L/(space/d)
Campgrounds	225	L/(campsite/d)
Mobile Home Parks	1,000	L/(Space/d)
Motels	150	L/(bed-space/d)
Hotels	225	L/(bed-space/d)
Tourist Commercial	28,000	L/gross ha/d
Other Commercial	28,000	L/gross ha/d
AVERAGE DAILY DEMAND	Residential	0.29 L/s
	Commercial/Industrial/Institutional	0.00 L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	9.5	x avg. day L/c/d
Industrial	1.5	x avg. day L/gross ha/d
Commercial	1.5	x avg. day L/gross ha/d
Institutional	1.5	x avg. day L/gross ha/d
MAXIMUM DAILY DEMAND	Residential	2.74 L/s
	Commercial/Industrial/Institutional	0.00 L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	14.3	x avg. day L/c/d
Industrial	1.8	x max. day L/gross ha/d
Commercial	1.8	x max. day L/gross ha/d
Institutional	1.8	x max. day L/gross ha/d
MAXIMUM HOUR DEMAND	Residential	4.12 L/s
	Commercial/Industrial/Institutional	0.00 L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.29	L/s
MAXIMUM DAILY DEMAND	2.74	L/s
MAXIMUM HOUR DEMAND	4.12	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Water Demands - BLDG B

Project:	355 Franktown
Project No.:	CCO-22-0402
Designed By:	CH
Checked By:	BSC
Date:	September 1, 2023
Site Area:	0.50 gross ha

Residential	NUMBER OF UNITS		UNIT RATE	
Single Family		homes	3.4	persons/unit
Semi-detached		homes	2.7	persons/unit
Townhouse		homes	2.7	persons/unit
Bachelor Apartment		units	1.4	persons/unit
1 Bedroom Apartment	18	units	1.4	persons/unit
2 Bedroom Apartment	30	units	2.1	persons/unit
3 Bedroom Apartment		units	3.1	persons/unit
Average Apartment		units	1.8	persons/unit
Total Population	89	persons		
Commercial		m2		
Industrial - Light		m2		
Industrial - Heavy		m2		

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	280	L/c/d
Industrial - Light	35,000	L/gross ha/d
Industrial - Heavy	55,000	L/gross ha/d
Shopping Centres	2,500	L/(1000m ² /d)
Hospital	900	L/(bed/day)
Schools	70	L/(Student/d)
Trailer Park with no Hook-Ups	340	L/(space/d)
Trailer Park with Hook-Ups	800	L/(space/d)
Campgrounds	225	L/(campsite/d)
Mobile Home Parks	1,000	L/(Space/d)
Motels	150	L/(bed-space/d)
Hotels	225	L/(bed-space/d)
Tourist Commercial	28,000	L/gross ha/d
Other Commercial	28,000	L/gross ha/d
AVERAGE DAILY DEMAND	Residential	0.29 L/s
	Commercial/Industrial/Institutional	0.00 L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	9.5	x avg. day
Industrial	1.5	x avg. day
Commercial	1.5	x avg. day
Institutional	1.5	x avg. day
MAXIMUM DAILY DEMAND	Residential	2.74 L/s
	Commercial/Industrial/Institutional	0.00 L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	14.3	x avg. day
Industrial	1.8	x max. day
Commercial	1.8	x max. day
Institutional	1.8	x max. day
MAXIMUM HOUR DEMAND	Residential	4.12 L/s
	Commercial/Industrial/Institutional	0.00 L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.29	L/s
MAXIMUM DAILY DEMAND	2.74	L/s
MAXIMUM HOUR DEMAND	4.12	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Water Demands - Existing Mall

Project:	355 Franktown
Project No.:	CCO-22-0402
Designed By:	CH
Checked By:	BSC
Date:	September 1, 2023
Site Area:	0.73 gross ha

Residential	NUMBER OF UNITS		UNIT RATE	
Single Family		homes	3.4	persons/unit
Semi-detached		homes	2.7	persons/unit
Townhouse		homes	2.7	persons/unit
Bachelor Apartment		units	1.4	persons/unit
1 Bedroom Apartment		units	1.4	persons/unit
2 Bedroom Apartment		units	2.1	persons/unit
3 Bedroom Apartment		units	3.1	persons/unit
Average Apartment		units	1.8	persons/unit

Total Population **0 persons**

Commercial	7299	m2
Industrial - Light		m2
Industrial - Heavy		m2

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	280	L/c/d
Industrial - Light	35,000	L/gross ha/d
Industrial - Heavy	55,000	L/gross ha/d
Shopping Centres	2,500	L/(1000m ² /d)
Hospital	900	L/(bed/day)
Schools	70	L/(Student/d)
Trailer Park with no Hook-Ups	340	L/(space/d)
Trailer Park with Hook-Ups	800	L/(space/d)
Campgrounds	225	L/(campsite/d)
Mobile Home Parks	1,000	L/(Space/d)
Motels	150	L/(bed-space/d)
Hotels	225	L/(bed-space/d)
Tourist Commercial	28,000	L/gross ha/d
Other Commercial	28,000	L/gross ha/d
AVERAGE DAILY DEMAND	Residential	0.00
	Commercial/Industrial/Institutional	0.24
		L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	9.5	x avg. day
Industrial	1.5	x avg. day
Commercial	1.5	x avg. day
Institutional	1.5	x avg. day
MAXIMUM DAILY DEMAND	Residential	0.00
	Commercial/Industrial/Institutional	0.35
		L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	14.3	x avg. day
Industrial	1.8	x max. day
Commercial	1.8	x max. day
Institutional	1.8	x max. day
MAXIMUM HOUR DEMAND	Residential	0.00
	Commercial/Industrial/Institutional	0.64
		L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.24	L/s
MAXIMUM DAILY DEMAND	0.35	L/s
MAXIMUM HOUR DEMAND	0.64	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Water Demands - Heafey Townhouse

Project:	355 Franktown
Project No.:	CCO-22-0402
Designed By:	CH
Checked By:	BSC
Date:	September 1, 2023
Site Area:	0.35 gross ha

Residential	NUMBER OF UNITS		UNIT RATE	
Single Family		homes	3.4	persons/unit
Semi-detached		homes	2.7	persons/unit
Townhouse	6	homes	2.7	persons/unit
Bachelor Apartment		units	1.4	persons/unit
1 Bedroom Apartment		units	1.4	persons/unit
2 Bedroom Apartment		units	2.1	persons/unit
3 Bedroom Apartment		units	3.1	persons/unit
Average Apartment		units	1.8	persons/unit

Total Population **17 persons**

Commercial	m2
Industrial - Light	m2
Industrial - Heavy	m2

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	280	L/c/d
Industrial - Light	35,000	L/gross ha/d
Industrial - Heavy	55,000	L/gross ha/d
Shopping Centres	2,500	L/(1000m ² /d)
Hospital	900	L/(bed/day)
Schools	70	L/(Student/d)
Trailer Park with no Hook-Ups	340	L/(space/d)
Trailer Park with Hook-Ups	800	L/(space/d)
Campgrounds	225	L/(campsite/d)
Mobile Home Parks	1,000	L/(Space/d)
Motels	150	L/(bed-space/d)
Hotels	225	L/(bed-space/d)
Tourist Commercial	28,000	L/gross ha/d
Other Commercial	28,000	L/gross ha/d
AVERAGE DAILY DEMAND	Residential	0.06 L/s
	Commercial/Industrial/ Institutional	0.00 L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	9.5	x avg. day L/c/d
Industrial	1.5	x avg. day L/gross ha/d
Commercial	1.5	x avg. day L/gross ha/d
Institutional	1.5	x avg. day L/gross ha/d
MAXIMUM DAILY DEMAND	Residential	0.52 L/s
	Commercial/Industrial/ Institutional	0.00 L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS
Residential	14.3	x avg. day L/c/d
Industrial	1.8	x max. day L/gross ha/d
Commercial	1.8	x max. day L/gross ha/d
Institutional	1.8	x max. day L/gross ha/d
MAXIMUM HOUR DEMAND	Residential	0.79 L/s
	Commercial/Industrial/ Institutional	0.00 L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.06	L/s
MAXIMUM DAILY DEMAND	0.52	L/s
MAXIMUM HOUR DEMAND	0.79	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Water Demands - Heafey Total

Project:	355 Franktown
Project No.:	CCO-22-0402
Designed By:	CH
Checked By:	BSC
Date:	September 1, 2023
Site Area:	2.07 gross ha

Residential	NUMBER OF UNITS		UNIT RATE	
Single Family		homes	3.4	persons/unit
Semi-detached		homes	2.7	persons/unit
Townhouse	6	homes	2.7	persons/unit
Bachelor Apartment		units	1.4	persons/unit
1 Bedroom Apartment	36	units	1.4	persons/unit
2 Bedroom Apartment	60	units	2.1	persons/unit
3 Bedroom Apartment		units	3.1	persons/unit
Average Apartment		units	1.8	persons/unit
Total Population		193	persons	
Commercial	7299	m2		
Industrial - Light		m2		
Industrial - Heavy		m2		

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	
Residential	280	L/c/d	
Industrial - Light	35,000	L/gross ha/d	
Industrial - Heavy	55,000	L/gross ha/d	
Shopping Centres	2,500	L/(1000m ² /d)	
Hospital	900	L/(bed/day)	
Schools	70	L/(Student/d)	
Trailer Park with no Hook-Ups	340	L/(space/d)	
Trailer Park with Hook-Ups	800	L/(space/d)	
Campgrounds	225	L/(campsite/d)	
Mobile Home Parks	1,000	L/(Space/d)	
Motels	150	L/(bed-space/d)	
Hotels	225	L/(bed-space/d)	
Tourist Commercial	28,000	L/gross ha/d	
Other Commercial	28,000	L/gross ha/d	
AVERAGE DAILY DEMAND	Residential Commerical/Industrial/ Institutional	0.63 0.24	L/s L/s

MAXIMUM DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	
Residential	4.9	x avg. day	
Industrial	1.5	x avg. day	
Commercial	1.5	x avg. day	
Institutional	1.5	x avg. day	
MAXIMUM DAILY DEMAND	Residential Commerical/Industrial/ Institutional	3.06 0.35	L/s L/s

MAXIMUM HOUR DEMAND

DEMAND TYPE	AMOUNT	UNITS	
Residential	7.4	x avg. day	
Industrial	1.8	x max. day	
Commercial	1.8	x max. day	
Institutional	1.8	x max. day	
MAXIMUM HOUR DEMAND	Residential Commerical/Industrial/ Institutional	4.63 0.64	L/s L/s

WATER DEMAND DESIGN FLOWS PER UNIT COUNT
CITY OF OTTAWA - WATER DISTRIBUTION GUIDELINES, JULY 2010

AVERAGE DAILY DEMAND	0.86	L/s
MAXIMUM DAILY DEMAND	3.42	L/s
MAXIMUM HOUR DEMAND	5.27	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Fire Underwriters Survey - Building A

Project: 355 Franktown
 Project No.: CCO-22-0402
 Designed By: CH
 Checked By: BSC
 Date: March 13, 2024

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.:
 City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

$F = 220 \times C \times \sqrt{A}$ Where:
 F = Required fire flow in liters per minute
 C = Coefficient related to the type of construction.
 A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in the building being considered.

Construction Type Ordinary Construction

C	1	A	4,140.0 m ²
Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area)			4,140.0 m ² *Unprotected Vertical Openings

Calculated Fire Flow	14,155.4 L/min
	14,000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:
 Limited Combustible -15%

Fire Flow	11,900.0 L/min
-----------	----------------

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Standard Water Supply Sprinklered -40%

Reduction	-4,760.0 L/min
-----------	----------------

D. INCREASE FOR EXPOSURE (No Rounding)

Exposure	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor	% Increase
Exposure 1	10.1 to 20	Ordinary - Mass Timber (Unprotected)	25	2	50.5	7%
Exposure 2	10.1 to 20	Ordinary - Mass Timber (Unprotected)	22	4	88.0	0% *Both Buildings Sprinklered
Exposure 3	Over 30 m	Ordinary - Mass Timber (Unprotected)	15	2	30.0	0%
Exposure 4	10.1 to 20	Ordinary - Mass Timber (Unprotected)	34	1	34.0	6%
						% Increase* 13%

Increase*	1,547.0 L/min
-----------	---------------

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow	8,687.0 L/min
Fire Flow Required**	9,000.0 L/min

*In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

**In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Fire Underwriters Survey - Building B

Project: 355 Franktown
 Project No.: CCO-22-0402
 Designed By: CH
 Checked By: BSC
 Date: March 13, 2024

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.:
 City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

F = 220 x C x vA Where: F = Required fire flow in liters per minute
 C = Coefficient related to the type of construction.
 A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in the building being considered.

Construction Type Ordinary Construction

C 1 A 4,140.0 m²
 Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area) 4,140.0 m² *Unprotected Vertical Openings

Calculated Fire Flow	14,155.4 L/min 14,000.0 L/min
----------------------	----------------------------------

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:
 Limited Combustible -15%

Fire Flow	11,900.0 L/min
-----------	----------------

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Standard Water Supply Sprinklered -40%

Reduction	-4,760.0 L/min
-----------	----------------

D. INCREASE FOR EXPOSURE (No Rounding)

Exposure	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor	
Exposure 1	Over 30 m	Ordinary - Mass Timber (Unprotected)	16	2	32.0	0%
Exposure 2	Over 30 m	Wood frame	38	2	76.0	0%
Exposure 3	Over 30 m	Ordinary - Mass Timber (Unprotected)	15	2	30.0	0%
Exposure 4	10.1 to 20	Ordinary - Mass Timber (Unprotected)	19	4	76.0	0% *Both Buildings Sprinklered
% Increase*						0%

Increase*	0.0 L/min
-----------	-----------

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow	7,140.0 L/min
Fire Flow Required**	7,000.0 L/min

*In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%

**In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Fire Underwriters Survey - Townhouse Block

Project: 355 Franktown
 Project No.: CCO-22-0402
 Designed By: CH
 Checked By: BSC
 Date: March 13, 2024

From the Fire Underwriters Survey (2020)

From Part II – Guide for Determination of Required Fire Flow Copyright I.S.O.:
 City of Ottawa Technical Bulletin ISTB-2018-02 Applied Where Applicable

A. BASE REQUIREMENT (Rounded to the nearest 1000 L/min)

$F = 220 \times C \times \sqrt{A}$ Where:
 F = Required fire flow in liters per minute
 C = Coefficient related to the type of construction.
 A = The total floor area in square meters (including all storey's, but excluding basements at least 50 percent below grade) in the building being considered.

Construction Type **Wood Frame**

C	1.5	A	1,132.0 m ²
Total Floor Area (per the 2020 FUS Page 20 - Total Effective Area)			1,132.0 m ² *Unprotected Vertical Openings

Calculated Fire Flow	11,102.9 L/min
	11,000.0 L/min

B. REDUCTION FOR OCCUPANCY TYPE (No Rounding)

From Page 24 of the Fire Underwriters Survey:
 Limited Combustible -15%

Fire Flow	9,350.0 L/min
-----------	---------------

C. REDUCTION FOR SPRINKLER TYPE (No Rounding)

Non-Sprinklered 0%

Reduction	0.0 L/min
-----------	-----------

D. INCREASE FOR EXPOSURE (No Rounding)

	Separation Distance (m)	Cons.of Exposed Wall	Length Exposed Adjacent Wall (m)	Height (Stories)	Length-Height Factor	
Exposure 1	3.1 to 10	Wood frame	15	2	30.0	16%
Exposure 2	20.1 to 30	Wood frame	29	2	58.0	4%
Exposure 3	Over 30 m	Ordinary - Mass Timber (Unprotected)	20	2	40.0	0%
Exposure 4	Over 30 m	Ordinary - Mass Timber (Unprotected)	22	4	88.0	0%
						% Increase* 20%

Increase*	1,870.0 L/min
-----------	---------------

E. Total Fire Flow (Rounded to the Nearest 1000 L/min)

Fire Flow	11,220.0 L/min
Fire Flow Required**	11,000.0 L/min

*In accordance with Part II, Section 4, the Increase for separation distance is not to exceed 75%
 **In accordance with Section 4 the Fire flow is not to exceed 45,000 L/min or be less than 2,000 L/min

Coleman Phase 2 & 355 Franktown Water Model

Average Day Demands

Junction Table - Time: 0.00 hours

ID	Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (psi)
31	J-2	133.92	0.00	181.14	67
32	J-3	133.31	0.07	181.16	68
34	J-4	133.50	0.12	181.16	68
36	J-5	133.19	0.39	181.17	68
38	J-6	133.35	0.10	181.17	68
40	J-7	133.35	0.10	181.18	68
42	J-8	133.06	0.04	181.18	68
44	J-9	133.12	0.03	181.19	68
46	J-10	133.10	0.00	181.20	68
48	J-11	133.26	0.00	181.16	68
50	J-12	133.27	0.00	181.17	68
52	J-13	130.65	0.00	181.18	72
54	J-14	133.56	0.11	181.15	68
88	J-15	133.31	0.00	181.21	68
90	J-16	133.91	0.58	181.23	67
92	J-17	134.88	0.06	181.21	66
95	J-18	136.18	0.00	181.29	64
97	J-19	136.41	0.25	181.25	64

Reservoir Table - Time: 0.00 hours

ID	Label	Elevation (m)	Hydraulic Grade (m)
57	R-1	181.14	181.14
58	R-2	181.16	181.16
59	R-3	181.18	181.18
94	R-4	181.32	181.32

Pipe Table - Time: 0.00 hours

ID	Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
35	P-2	62	J-3	J-4	204.0	PVC	110.0	-2.38	0.07
37	P-3	72	J-4	J-5	204.0	PVC	110.0	-2.50	0.08
39	P-4	75	J-5	J-6	204.0	PVC	110.0	-2.89	0.09
41	P-5	87	J-6	J-7	204.0	PVC	110.0	-2.99	0.09
43	P-6	47	J-7	J-8	204.0	PVC	110.0	-3.09	0.09
45	P-7	31	J-8	J-9	204.0	PVC	110.0	-5.01	0.15
47	P-8	35	J-9	J-10	204.0	PVC	110.0	-5.04	0.15
49	P-9	12	J-11	J-3	204.0	PVC	110.0	1.96	0.06
51	P-10	45	J-12	J-5	204.0	PVC	110.0	0.00	0.00
53	P-11	103	J-13	J-8	204.0	PVC	110.0	-1.88	0.06
55	P-1(1)	50	J-2	J-14	204.0	PVC	110.0	-4.16	0.13

Coleman Phase 2 & 355 Franktown Water Model

Average Day Demands

Pipe Table - Time: 0.00 hours

ID	Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen- Williams C	Flow (L/s)	Velocity (m/s)
56	P-1(2)	47	J-14	J-3	204.0	PVC	110.0	-4.27	0.13
60	P-12	20	R-1	J-2	204.0	PVC	110.0	-4.16	0.13
61	P-13	19	R-2	J-11	204.0	PVC	110.0	1.96	0.06
62	P-14	23	R-3	J-13	204.0	PVC	110.0	-1.88	0.06
89	P-16	45	J-10	J-15	204.0	PVC	110.0	-5.04	0.15
91	P-17	80	J-15	J-16	204.0	PVC	110.0	-5.10	0.16
93	P-18	48	J-15	J-17	204.0	PVC	110.0	0.06	0.00
96	P-19	111	R-4	J-18	204.0	PVC	110.0	5.93	0.18
98	P-20	130	J-18	J-19	204.0	PVC	110.0	5.93	0.18
99	P-21	66	J-19	J-16	204.0	PVC	110.0	5.68	0.17

Coleman Phase 2 & 355 Franktown Water Model

Peak Hour Demands

Junction Table - Time: 0.00 hours

ID	Label	Elevation (m)	Demand (L/s)	Hydraulic Grade (m)	Pressure (psi)
31	J-2	133.92	0.00	181.04	67
32	J-3	133.31	1.00	180.95	68
34	J-4	133.50	1.72	180.90	67
36	J-5	133.19	0.39	180.87	68
38	J-6	133.35	1.43	180.83	67
40	J-7	133.35	1.43	180.80	67
42	J-8	133.06	0.57	180.79	68
44	J-9	133.12	0.43	180.78	68
46	J-10	133.10	0.00	180.76	68
48	J-11	133.26	0.00	180.95	68
50	J-12	133.27	0.00	180.87	68
52	J-13	130.65	0.00	180.81	71
54	J-14	133.56	1.57	180.99	67
88	J-15	133.31	0.00	180.74	67
90	J-16	133.91	8.15	180.70	66
92	J-17	134.88	0.06	180.74	65
95	J-18	136.18	0.00	180.72	63
97	J-19	136.41	3.58	180.70	63

Reservoir Table - Time: 0.00 hours

ID	Label	Elevation (m)	Hydraulic Grade (m)
57	R-1	181.06	181.06
58	R-2	180.95	180.95
59	R-3	180.81	180.81
94	R-4	180.74	180.74

Pipe Table - Time: 0.00 hours

ID	Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
35	P-2	62	J-3	J-4	204.0	PVC	110.0	9.64	0.29
37	P-3	72	J-4	J-5	204.0	PVC	110.0	7.92	0.24
39	P-4	75	J-5	J-6	204.0	PVC	110.0	7.53	0.23
41	P-5	87	J-6	J-7	204.0	PVC	110.0	6.10	0.19
43	P-6	47	J-7	J-8	204.0	PVC	110.0	4.67	0.14
45	P-7	31	J-8	J-9	204.0	PVC	110.0	7.99	0.24
47	P-8	35	J-9	J-10	204.0	PVC	110.0	7.56	0.23
49	P-9	12	J-11	J-3	204.0	PVC	110.0	0.67	0.02
51	P-10	45	J-12	J-5	204.0	PVC	110.0	0.00	0.00
53	P-11	103	J-13	J-8	204.0	PVC	110.0	3.89	0.12
55	P-1(1)	50	J-2	J-14	204.0	PVC	110.0	11.54	0.35

Coleman Phase 2 & 355 Franktown Water Model

Peak Hour Demands

Pipe Table - Time: 0.00 hours

ID	Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen- Williams C	Flow (L/s)	Velocity (m/s)
56	P-1(2)	47	J-14	J-3	204.0	PVC	110.0	9.97	0.30
60	P-12	20	R-1	J-2	204.0	PVC	110.0	11.54	0.35
61	P-13	19	R-2	J-11	204.0	PVC	110.0	0.67	0.02
62	P-14	23	R-3	J-13	204.0	PVC	110.0	3.89	0.12
89	P-16	45	J-10	J-15	204.0	PVC	110.0	7.56	0.23
91	P-17	80	J-15	J-16	204.0	PVC	110.0	7.50	0.23
93	P-18	48	J-15	J-17	204.0	PVC	110.0	0.06	0.00
96	P-19	111	R-4	J-18	204.0	PVC	110.0	4.23	0.13
98	P-20	130	J-18	J-19	204.0	PVC	110.0	4.23	0.13
99	P-21	66	J-19	J-16	204.0	PVC	110.0	0.65	0.02

Coleman Phase 2 & 355 Franktown Water Model
 Max Day + Fire Flow, Reduced HGL (Min. 190L/sec)
 Fire Flow Results Table - Time: 0.00 hours

Label	Fire Flow (Available) (L/s)	Pressure (Calculated Residual) (psi)	Junction w/ Minimum Pressure (Zone)	Pipe w/ Maximum Velocity	Velocity of Maximum Pipe (m/s)	Satisfies Fire Flow Constraints ?
J-2	300.00	39	J-19	P-12	6.61	True
J-3	300.00	42	J-19	P-9	6.36	True
J-4	300.00	23	J-12	P-2	6.56	True
J-5	261.30	20	J-12	P-2	4.80	True
J-6	251.51	20	J-12	P-6	4.05	True
J-7	283.80	20	J-6	P-6	5.63	True
J-8	300.00	27	J-9	P-11	4.85	True
J-9	289.81	20	J-10	P-7	6.53	True
J-10	255.20	20	J-17	P-7	5.37	True
J-11	300.00	46	J-19	P-13	8.48	True
J-12	187.39	20	J-5	P-10	5.73	False
J-13	300.00	49	J-19	P-14	8.85	True
J-14	300.00	35	J-19	P-1(2)	4.80	True
J-15	225.54	20	J-17	P-7	4.39	True
J-16	201.37	20	J-19	P-7	3.43	True
J-17	168.32	20	J-15	P-18	5.17	False
J-18	233.94	20	J-19	P-19	4.94	True
J-19	191.46	20	J-16	P-20	3.23	True

Reservoir Table - Time: 0.00 hours

ID	Label	Elevation (m)	Hydraulic Grade (m)
57	R-1	166.06	166.06
58	R-2	165.95	165.95
59	R-3	165.81	165.81
94	R-4	165.74	165.74

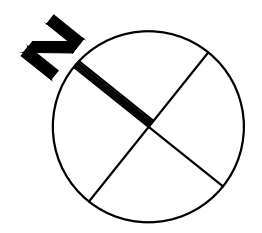
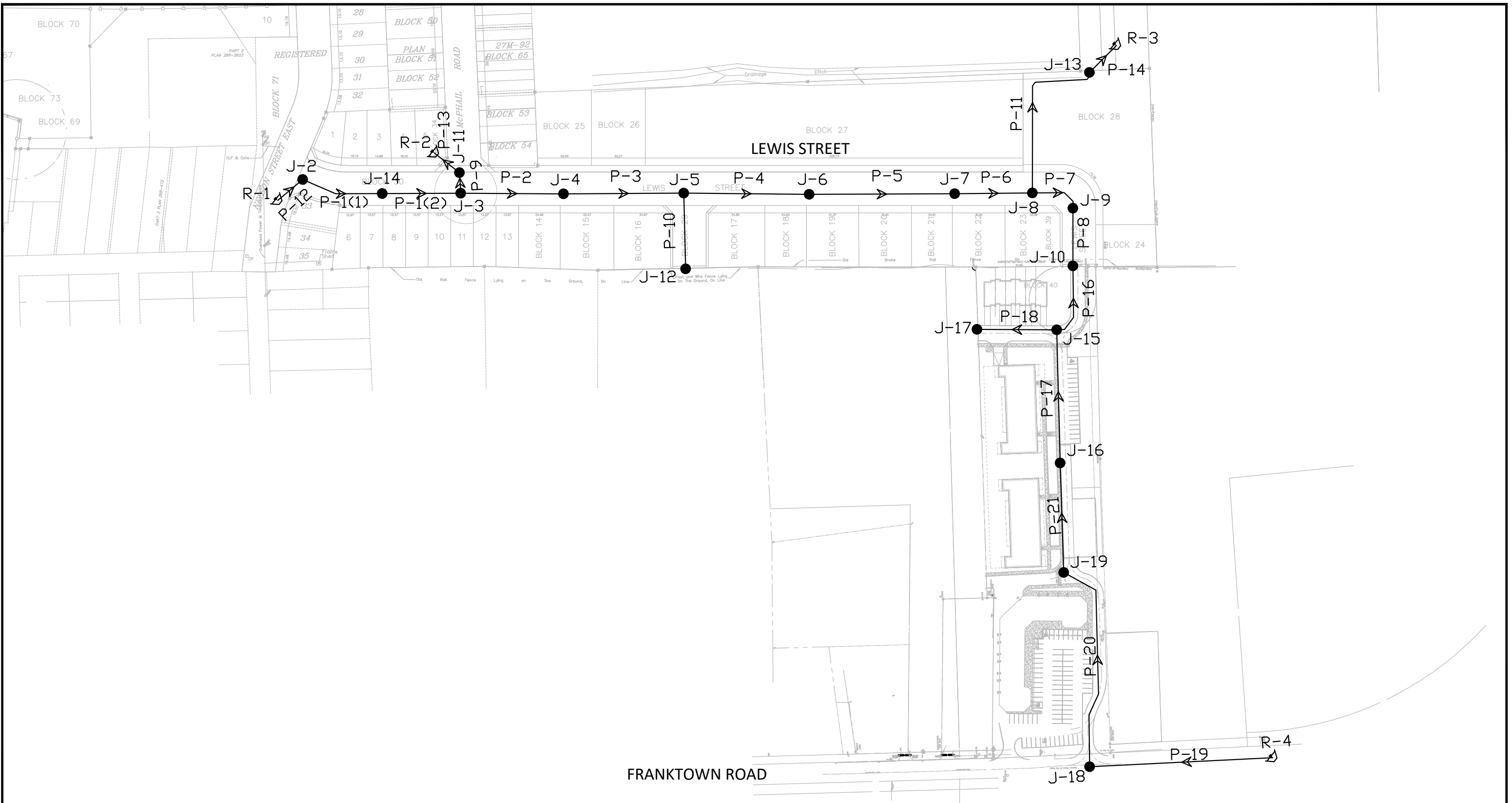
Pipe Table - Time: 0.00 hours

ID	Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen-Williams C	Flow (L/s)	Velocity (m/s)
35	P-2	62	J-3	J-4	204.0	PVC	110.0	10.53	0.32
37	P-3	72	J-4	J-5	204.0	PVC	110.0	9.39	0.29
39	P-4	75	J-5	J-6	204.0	PVC	110.0	5.68	0.17
41	P-5	87	J-6	J-7	204.0	PVC	110.0	4.73	0.14
43	P-6	47	J-7	J-8	204.0	PVC	110.0	3.78	0.12
45	P-7	31	J-8	J-9	204.0	PVC	110.0	6.84	0.21
47	P-8	35	J-9	J-10	204.0	PVC	110.0	6.55	0.20
49	P-9	12	J-11	J-3	204.0	PVC	110.0	0.90	0.03
51	P-10	45	J-12	J-5	204.0	PVC	110.0	0.00	0.00
53	P-11	103	J-13	J-8	204.0	PVC	110.0	3.44	0.11

Coleman Phase 2 & 355 Franktown Water Model
 Max Day + Fire Flow, Reduced HGL (Min. 190L/sec)
 Pipe Table - Time: 0.00 hours

ID	Label	Length (Scaled) (m)	Start Node	Stop Node	Diameter (mm)	Material	Hazen- Williams C	Flow (L/s)	Velocity (m/s)
55	P-1(1)	50	J-2	J-14	204.0	PVC	110.0	11.34	0.35
56	P-1(2)	47	J-14	J-3	204.0	PVC	110.0	10.29	0.31
60	P-12	20	R-1	J-2	204.0	PVC	110.0	11.34	0.35
61	P-13	19	R-2	J-11	204.0	PVC	110.0	0.90	0.03
62	P-14	23	R-3	J-13	204.0	PVC	110.0	3.44	0.11
89	P-16	45	J-10	J-15	204.0	PVC	110.0	6.55	0.20
91	P-17	80	J-15	J-16	204.0	PVC	110.0	5.98	0.18
93	P-18	48	J-15	J-17	204.0	PVC	110.0	0.57	0.02
96	P-19	111	R-4	J-18	204.0	PVC	110.0	1.81	0.06
98	P-20	130	J-18	J-19	204.0	PVC	110.0	1.81	0.06
99	P-21	66	J-19	J-16	204.0	PVC	110.0	-0.57	0.02

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Drawn by:	J.H.	Checked By:	B.C.
Scale:	N.T.S.	Project Number:	CCO-22-0402

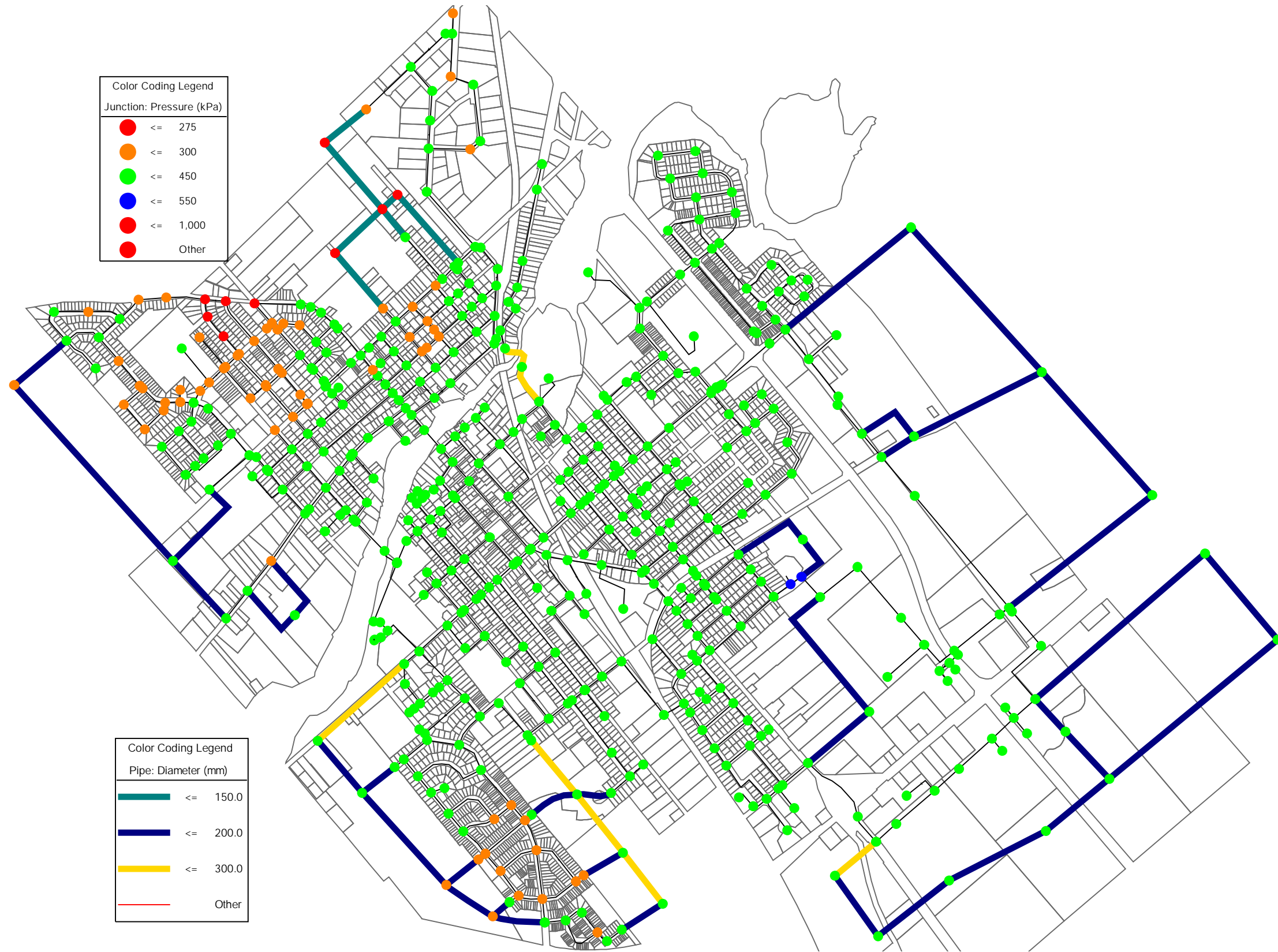
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 768 BOULEVARD SAINT-JOSEPH SUITE 100, GATINEAU, QUEBEC

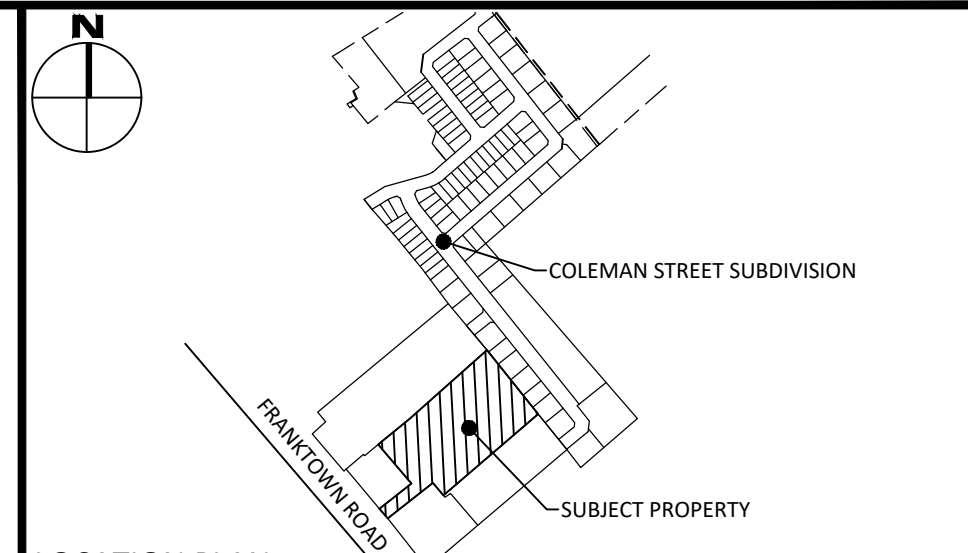
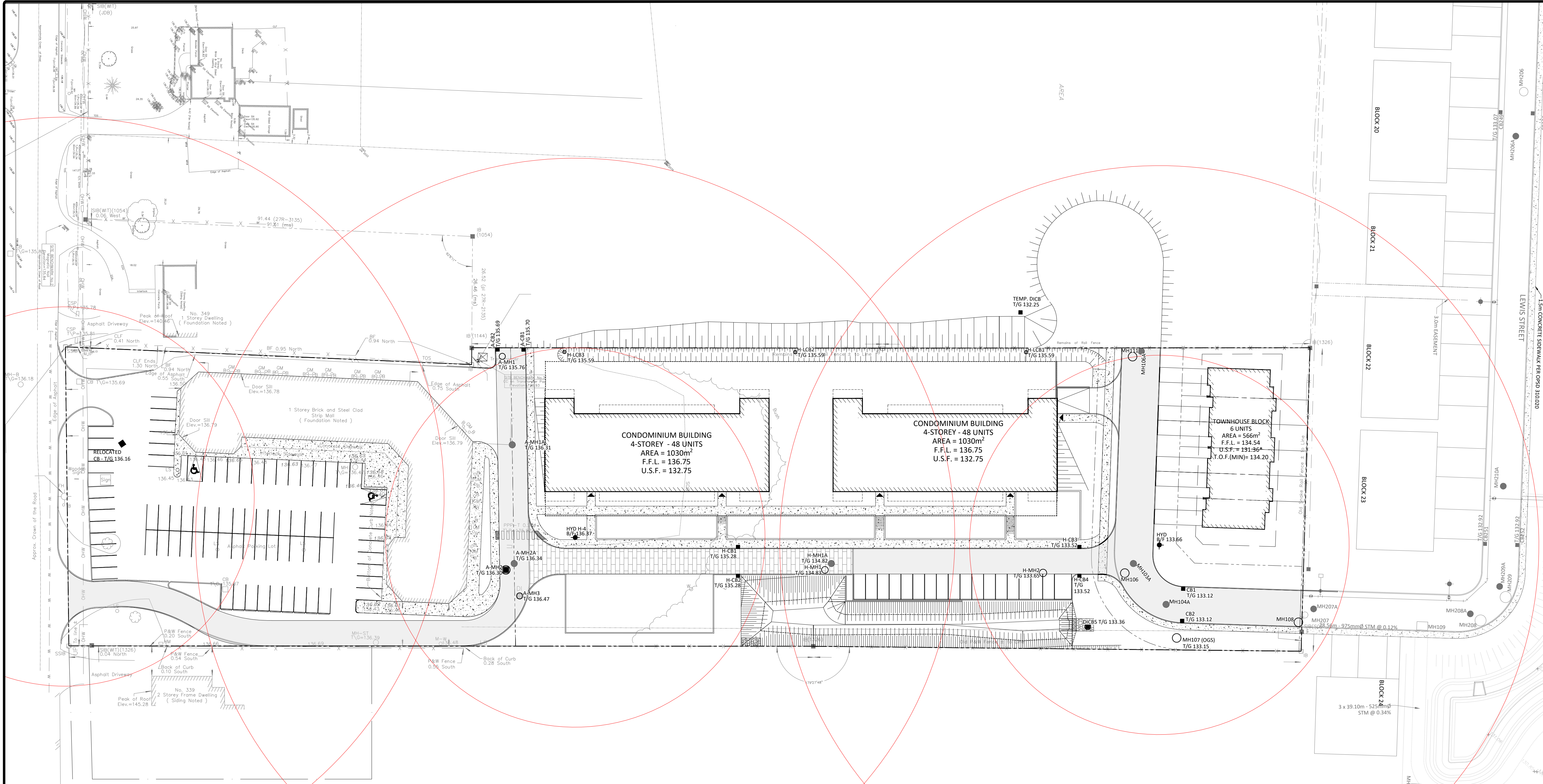
Project: **355 FRANKTOWN ROAD**
 CARLETON PLACE, ON

Drawing Title: **HYDRAULIC WATER MODEL**

No.	Revisions	Date	Drawing Number: C1
2	UPDATE LAYOUT	2024-MAR	
1	WATER MODEL	2023-AUG	

Active Scenario: Peak Hour - Build-out





LEGEND

— DC —	BARRIER CURB	— S —	SILT FENCE BARRIER (AS PER OPSD 219.130)
— M —	CURB DEPRESSION	— S —	STRAW BALE CHECK DAM (AS PER OPSD 219.180)
— E —	EASEMENT	— S —	SEDIMENT CONTROL DEVICE
— A —	ASPHALT	— S —	BUILDING ENTRANCE OVERHEAD DOOR
— R —	RETAINING WALL	— S —	REMOTE WATER METER
— C —	CONCRETE SIDEWALK	— S —	WATER METER
— P —	PAVING STONE	— S —	MISC. ROCK BOULDER
— M —	STORM MANHOLE	— S —	SEDIMENT CONTROL DEVICE
— C —	CATCHBASIN OR DITCH INLET	— S —	
— L —	LANDSCAPE CATCHBASIN	— S —	
— S —	SANITARY MANHOLE	— S —	
— P —	PERFORATED PIPE IN SWALES	— S —	
— W —	WATER VALVE/CHAMBER	— S —	
— F —	FIRE HYDRANT	— S —	
— S —	CENTRELINE OF SWALE	— S —	
— S —	SLOPING AT 3:1 (UNLESS SPECIFIED)	— S —	
— S —	PROPOSED ELEVATION	— S —	
— S —	EXISTING ELEVATION	— S —	
— S —	SWALE ELEVATION	— S —	
— S —	TOP OF WALL ELEVATION	— S —	
— S —	BOTTOM OF WALL ELEVATION	— S —	
— S —	EMERGENCY OVERLAND FLOW ROUTE	— S —	
— S —	AREA DRAIN	— S —	
— S —	METER/REMOTE METER	— S —	
— S —	DOWNSPOUT	— S —	

FOR REVIEW ONLY
NOT FOR CONSTRUCTION

No.	Revisions	Date
4	REVISED AS PER COMMENTS	JUNE 14, 2024
3	REVISED AS PER COMMENTS	SEPT. 1, 2023
2	REVISED SERVICING	AUG. 14, 2023
1	ISSUED FOR REVIEW	APR. 06, 2022

Check and verify all dimensions before proceeding with the work. Do not scale drawings.

SCALE 1 : 150

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Client:

Project:
355 FRANKTOWN ROAD
 CARLETON PLACE, ONTARIO

Drawing Title:
HYDRANT COVERAGE PLAN

Scale:	1:500	Project Number:	CCO-22-0402
Drawn By:	C.H.	Checked By:	B.C.
Designed By:	C.H.	Drawing Number:	HYD

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APPENDIX D
SANITARY CALCULATIONS

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Sanitary Demands - Areas S1, S2 & S3

Project:	355 Franktown		
Project No.:	CCO-22-0402		
Designed By:	CH		
Checked By:	BSC		
Date:	Mar-24		
Site Area	2.99	Gross ha	
1 Bedroom	207	1.40	Persons per unit
2 Bedroom	15	2.10	Persons per unit
Townhouse	18	2.70	Persons per unit
Total Population	370	Persons	
Commercial Area	1144	m ²	
Amenity Space		m ²	

DESIGN PARAMETERS

Institutional/Commercial Peaking Factor	1.5	*Check technical bulleting (Either use 1.0 or 1.5)
Residential Peaking Factor	3.43	* Using Harmon Formula = $1 + (14 / (4 + P^{0.5})) * 0.8$ where P = population in thousands, Harmon's Correction Factor = 0.8
Mannings coefficient (n)	0.013	
Demand (per capita)	280	L/day
Infiltration allowance	0.33	L/s/Ha

EXTRANEEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.15
Wet	0.84
Total	0.99

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	370	1.20
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)	1144.00	0.04
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	1.20	L/s
PEAK RESIDENTIAL FLOW	4.11	L/s
AVERAGE ICI FLOW	0.04	L/s
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.06	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.06	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	1.39	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	4.32	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	5.16	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Sanitary Demands -Condos - Area S5

Project:	355 Franktown		
Project No.:	CCO-22-0402		
Designed By:	CH		
Checked By:	BSC		
Date:	Mar-24		
Site Area	0.99	Gross ha	
1 Bedroom	36	1.40	Persons per unit
2 Bedroom	60	2.10	Persons per unit
Townhouse	0	2.70	Persons per unit
Total Population	177	Persons	
Commercial Area	0.00	m ²	
Amenity Space	0.00	m ²	

DESIGN PARAMETERS

Institutional/Commercial Peaking Factor	1.5	*Check technical bulleting (Either use 1.0 or 1.5)
Residential Peaking Factor	3.53	* Using Harmon Formula = $1+(14/(4+P^{0.5}))^{*0.8}$ where P = population in thousands, Harmon's Correction Factor = 0.8
Mannings coefficient (n)	0.013	
Demand (per capita)	280	L/day
Infiltration allowance	0.33	L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.05
Wet	0.28
Total	0.33

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	177	0.57
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)	0.00	0.00
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.57	L/s
PEAK RESIDENTIAL FLOW	2.03	L/s
AVERAGE ICI FLOW	0.00	L/s
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.00	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.00	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.62	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	2.08	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	2.35	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Sanitary Demands - Existing Mall - Area S6

Project:	355 Franktown	
Project No.:	CCO-22-0402	
Designed By:	CH	
Checked By:	BSC	
Date:	Mar-24	
Site Area	0.73	Gross ha
Total Population	0	Persons
Commercial Area	7288	m ²
Amenity Space	0.00	m ²

DESIGN PARAMETERS

Institutional/Commercial Peaking Factor	1.5	*Check technical bulleting (Either use 1.0 or 1.5)
Residential Peaking Factor	3.80	* Using Harmon Formula = $1+(14/(4+P^{0.5}))^{*0.8}$ where P = population in thousands, Harmon's Correction Factor = 0.8
Mannings coefficient (n)	0.013	
Demand (per capita)	280	L/day
Infiltration allowance	0.33	L/s/Ha

EXTRANEIOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.04
Wet	0.20
Total	0.24

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	0	0.00
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)	7288.00	0.24
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.00	L/s
PEAK RESIDENTIAL FLOW	0.00	L/s
AVERAGE ICI FLOW	0.24	L/s
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.35	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.35	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.27	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	0.39	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	0.60	L/s

McINTOSH PERRY

CCO-22-0402 - 355 Franktown - Sanitary Demands Total - Areas S4-S6

Project:	355 Franktown		
Project No.:	CCO-22-0402		
Designed By:	C.H.		
Checked By:	BSC		
Date:	Mar-24		
Site Area	2.07	Gross ha	
1 Bedroom	36	1.40	Persons per unit
2 Bedroom	60	2.10	Persons per unit
Apartment	0	1.80	Persons per unit
Townhouse	6	2.70	Persons per unit
Total Population	193	Persons	
Commercial Area	7288.0	m ²	

DESIGN PARAMETERS

Institutional/Commercial Peaking Factor	1.5	*Check technical bulleting (Either use 1.0 or 1.5)
Residential Peaking Factor	3.52	* Using Harmon Formula = $1+(14/(4+P^{0.5}))^{*0.8}$ where P = population in thousands, Harmon's Correction Factor = 0.8
Mannings coefficient (n)	0.013	
Demand (per capita)	280	L/day
Infiltration allowance	0.33	L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.10
Wet	0.58
Total	0.68

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	193	0.63
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)	7288.00	0.24
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	0.63	L/s
PEAK RESIDENTIAL FLOW	2.20	L/s
AVERAGE ICI FLOW	0.24	L/s
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.35	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.35	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	0.97	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	2.66	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	3.24	L/s

McINTOSH PERRY

CCO-22-0402 - 347 & 355 Franktown - Sanitary Demands Total - Areas S1-S6

Project:	355 Franktown		
Project No.:	CCO-22-0402		
Designed By:	CH		
Checked By:	BSC		
Date:	Mar-24		
Site Area	5.06	Gross ha	
1 Bedroom	243	1.40	Persons per unit
2 Bedroom	75	2.10	Persons per unit
Apartment	0	1.80	Persons per unit
Townhouse	24	2.70	Persons per unit
Total Population	563	Persons	
Commercial Area	8432.00	m ²	
Amenity Space		m ²	

DESIGN PARAMETERS

Institutional/Commercial Peaking Factor	1.5	*Check technical bulleting (Either use 1.0 or 1.5)
Residential Peaking Factor	3.36	* Using Harmon Formula = $1+(14/(4+P^{0.5}))^{0.8}$ where P = population in thousands, Harmon's Correction Factor = 0.8
Mannings coefficient (n)	0.013	
Demand (per capita)	280	L/day
Infiltration allowance	0.33	L/s/Ha

EXTRANEOUS FLOW ALLOWANCES

Infiltration / Inflow	Flow (L/s)
Dry	0.25
Wet	1.42
Total	1.67

AVERAGE DAILY DEMAND

DEMAND TYPE	AMOUNT	UNITS	POPULATION / AREA	Flow (L/s)
Residential	280	L/c/d	563	1.82
Industrial - Light**	35,000	L/gross ha/d		0
Industrial - Heavy**	55,000	L/gross ha/d		0
Commercial / Amenity	2,800	L/(1000m ² /d)	8432.00	0.27
Hospital	900	L/(bed/day)		0
Schools	70	L/(Student/d)		0
Trailer Parks no Hook-Ups	340	L/(space/d)		0
Trailer Park with Hook-Ups	800	L/(space/d)		0
Campgrounds	225	L/(campsite/d)		0
Mobile Home Parks	1,000	L/(Space/d)		0
Motels	150	L/(bed-space/d)		0
Hotels	225	L/(bed-space/d)		0
Office	75	L/7.0m ² /d		0
Tourist Commercial	28,000	L/gross ha/d		0
Other Commercial	28,000	L/gross ha/d		0

AVERAGE RESIDENTIAL FLOW	1.82	L/s
PEAK RESIDENTIAL FLOW	6.13	L/s
AVERAGE ICI FLOW	0.27	L/s
PEAK INSTITUTIONAL/COMMERCIAL FLOW	0.41	L/s
PEAK INDUSTRIAL FLOW	0.00	L/s
TOTAL PEAK ICI FLOW	0.41	L/s

TOTAL SANITARY DEMAND

TOTAL ESTIMATED AVERAGE DRY WEATHER FLOW	2.35	L/s
TOTAL ESTIMATED PEAK DRY WEATHER FLOW	6.79	L/s
TOTAL ESTIMATED PEAK WET WEATHER FLOW	8.21	L/s

SANITARY SEWER DESIGN SHEET

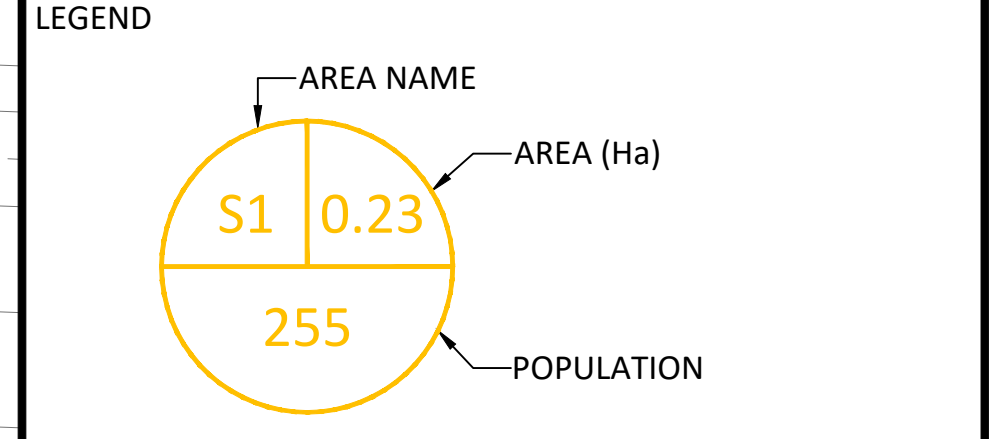
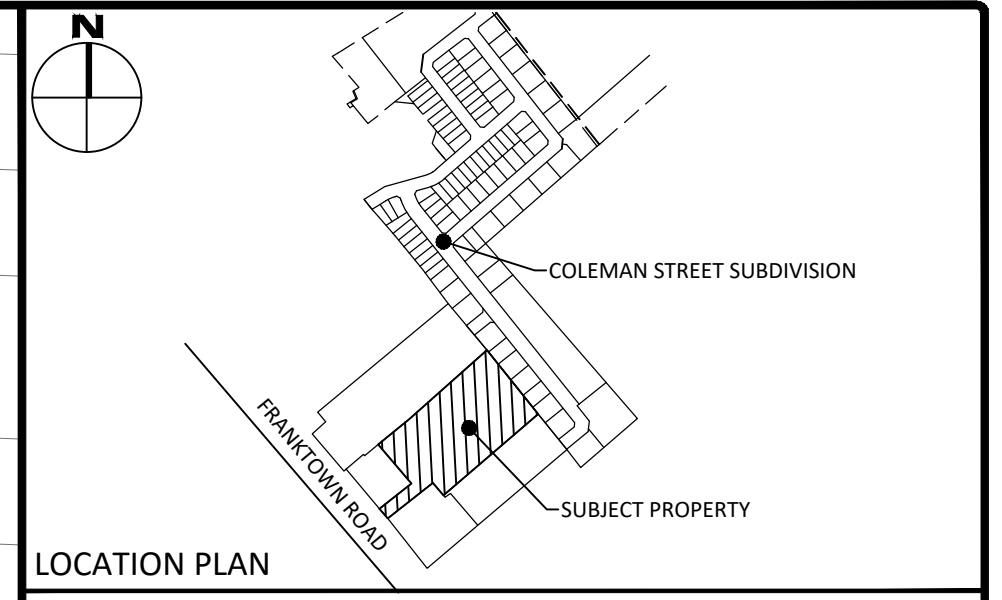
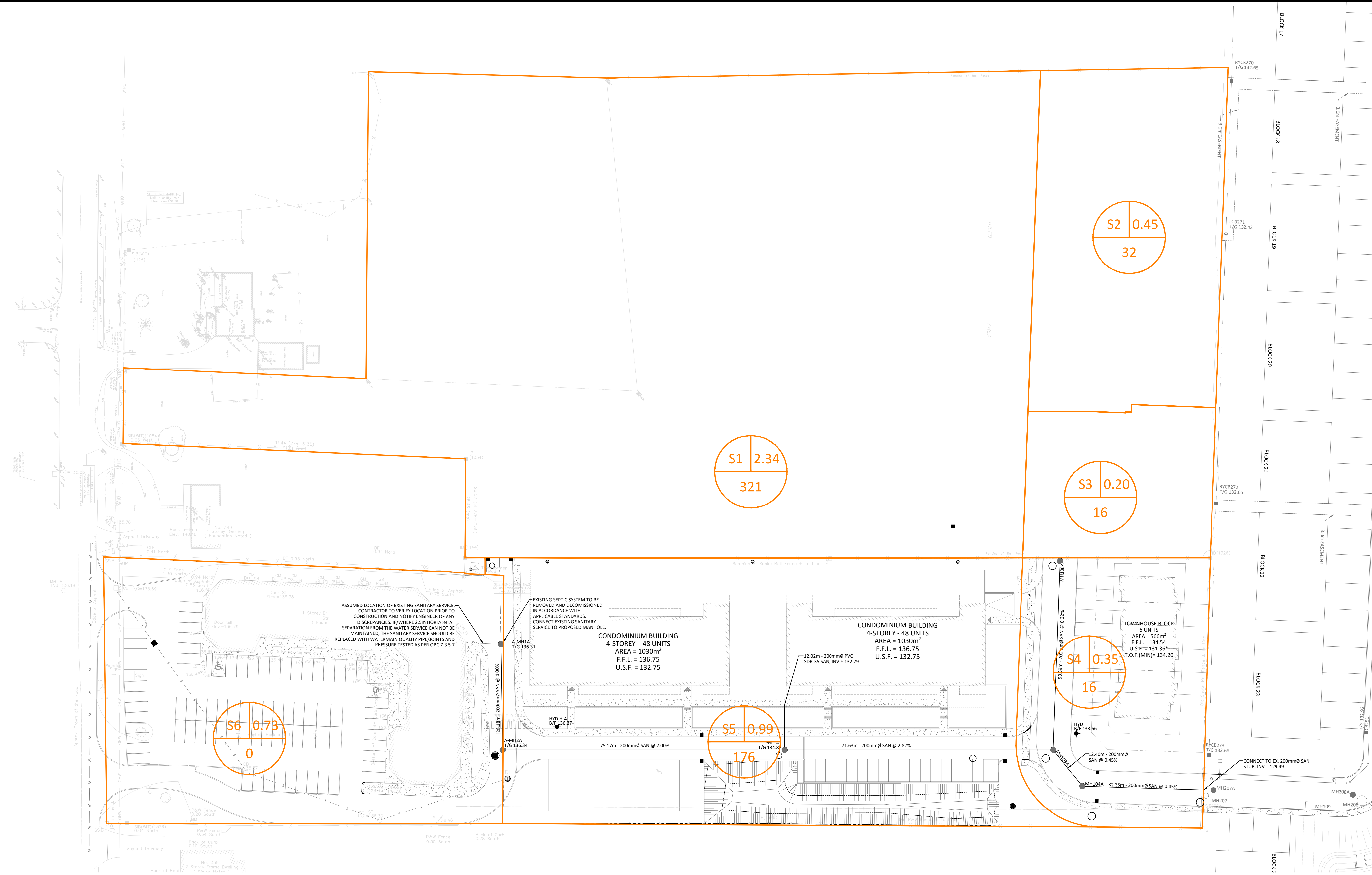
PROJECT: 355 Franktown Road
 LOCATION: Heafey Group
 CLIENT: Heafey Group



LOCATION				RESIDENTIAL									ICI AREAS						INFILTRATION ALLOWANCE			FLOW		SEWER DATA										
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31				
AREA ID	STREET	FROM MH	TO MH	UNIT TYPES				AREA (ha)	POPULATION		PEAK FACTOR	PEAK FLOW (L/s)	AREA (ha)						PEAK FLOW (L/s)	AREA (ha)		FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY					
				1-BED	2-BED	TH	APT		IND	CUM			INSTITUTIONAL		COMMERCIAL		INDUSTRIAL			IND	CUM								IND	CUM	L/s	(%)	L/s	(%)
													IND	CUM	IND	CUM	IND	CUM		IND	CUM								IND	CUM				
S1	Chadha Lands	MH101A	MH102A	207	15			2.34	321.3	321	3.45	3.59			0.11	0.11				0.06	2.34	2.34	0.77	4.42	19.36	129.34	200	0.32	0.597	14.93	77.15			
S2	Municipal Road	MH105A	MH102A			12		0.45	32.4	32	3.68	0.39									0.45	0.45	0.15	0.54	19.36	87.61	200	0.32	0.597	18.82	97.23			
S3	Municipal Road	MH102A	MH106A			6		0.20	16.2	370	3.43	4.11							0.06	0.20	2.99	0.99	5.16	19.36	90.35	200	0.32	0.597	14.20	73.36				
S4	Municipal Road	MH106A	MH103A			6		0.35	16.2	386	3.42	4.28							0.00	0.35	3.34	1.10	5.39	19.36	50.39	200	0.32	0.597	13.97	72.17				
S6	Mall Lands	A-MH1A	A-MH2A												0.73	0.73				0.35	0.73	0.73	0.24	0.59	34.22	28.18	200	1.00	1.055	33.62	98.26			
	Heafey Lands	A-MH2A	H-MH1A																0.35	0.00	0.73	0.24	0.60	48.39	75.17	200	2.00	1.492	47.79	98.77				
S5	Heafey Lands	H-MH1A	MH103A	36	60			0.99	176.4	176	3.53	2.02							0.35	0.99	1.72	0.57	2.94	57.46	71.63	200	2.82	1.772	54.52	94.88				
S1-S6	Municipal Road	MH103A	MH104A							563	3.36	6.12							0.41	0.00	5.06	1.67	8.20	22.95	12.40	200	0.45	0.708	14.75	64.27				
S1-S6	Municipal Road	MH104A	MH207A							563	3.36	6.12							0.41	0.00	5.06	1.67	8.20	22.95	32.74	200	0.45	0.708	14.75	64.27				

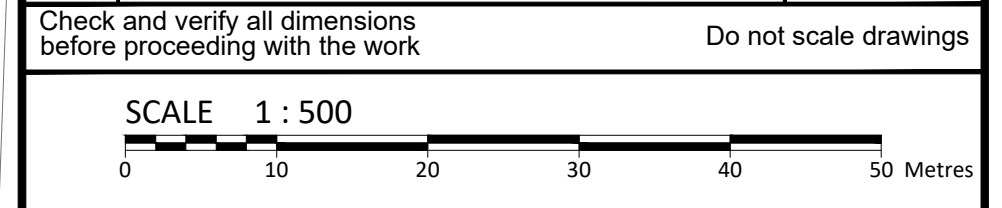
Design Parameters:				Notes:				Designed: RP				No.		Revision				Date	
Residential				ICI Areas				1. Mannings coefficient (n) = 0.013				1		ISSUED FOR REVIEW				2022-07-15	
1-BED 1.4 p/p/u				INST 28,000 L/Ha/day				2. Demand (per capita): 280 L/day				2		ISSUED FOR REVIEW				2023-03-02	
2-BED 2.1 p/p/u				COM 28,000 L/Ha/day				3. Infiltration allowance: 0.33 L/s/Ha				3		REVISED PER COMMENTS				2023.05.25	
TH 2.7 p/p/u				IND 35,000 L/Ha/day				4. Residential Peaking Factor:				4		ISSUED FOR REVIEW				2023.07.14	
Apt 1.8 p/p/u				MOE Chart				Harmon Formula = 1+(14/(4+P^0.5)*0.8)				5		REVISED PER COMMENTS				2023.09.01	
Other 60 p/p/Ha								where P = population in thousands				6		ISSUED FOR REVIEW				2024.03.06	
Project No.: CCO-22-0402																Sheet No:			
																1 of 1			

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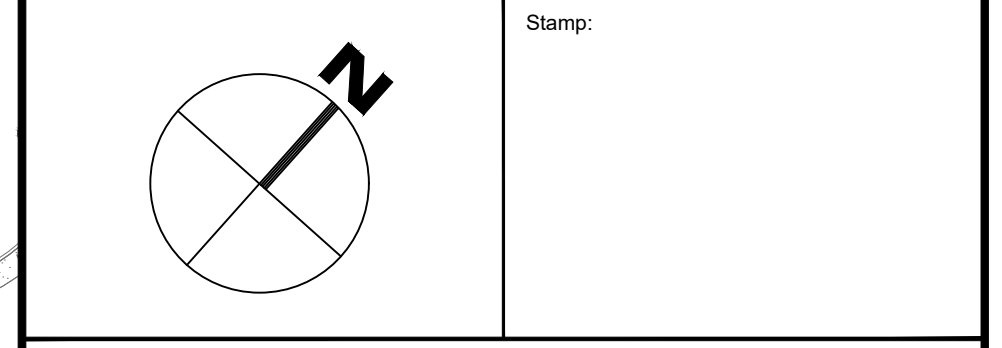


No.	Revisions	Date
5	ISSUED FOR REVIEW	JUNE 13, 2024
4	ISSUED FOR REVIEW	MAR. 08, 2024
3	REVISED AS PER COMMENTS	SEPT. 1, 2023
2	ISSUED FOR REVIEW	AUG. 11, 2023
1	ISSUED FOR REVIEW	MAR. 25, 2023

Check and verify all dimensions before proceeding with the work		
No.	Revisions	Date



McINTOSH PERRY
 115 Walgreen Road, RR3, Carp, ON K0A 1L0
 Tel: 613-836-2184 Fax: 613-836-3742
 www.mcintoshperry.com



Client: **11309455 CANADA INC**
 190 LISGAR ST,
 OTTAWA, ON K2P 0C4

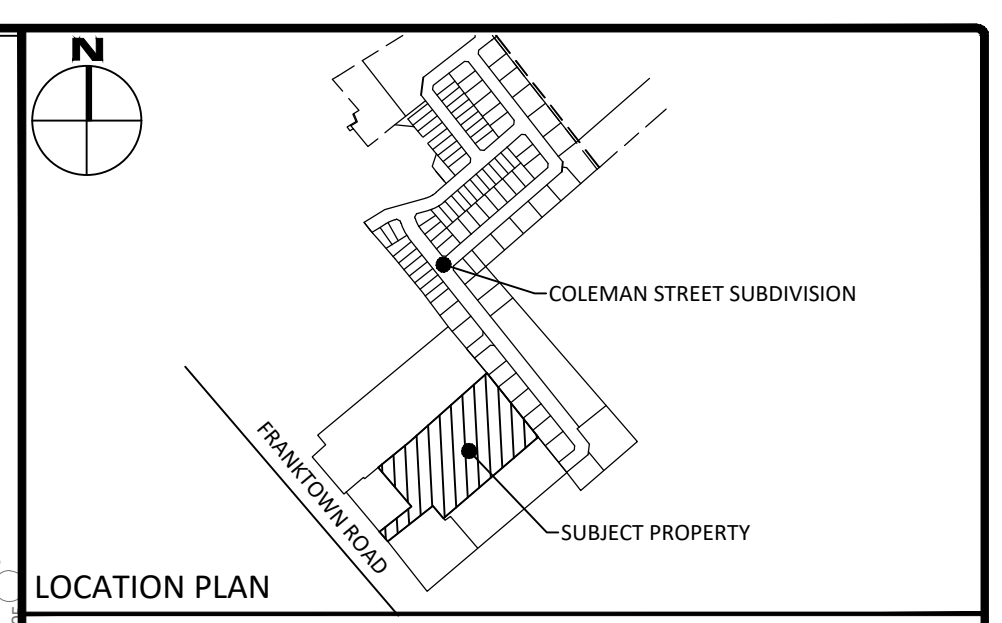
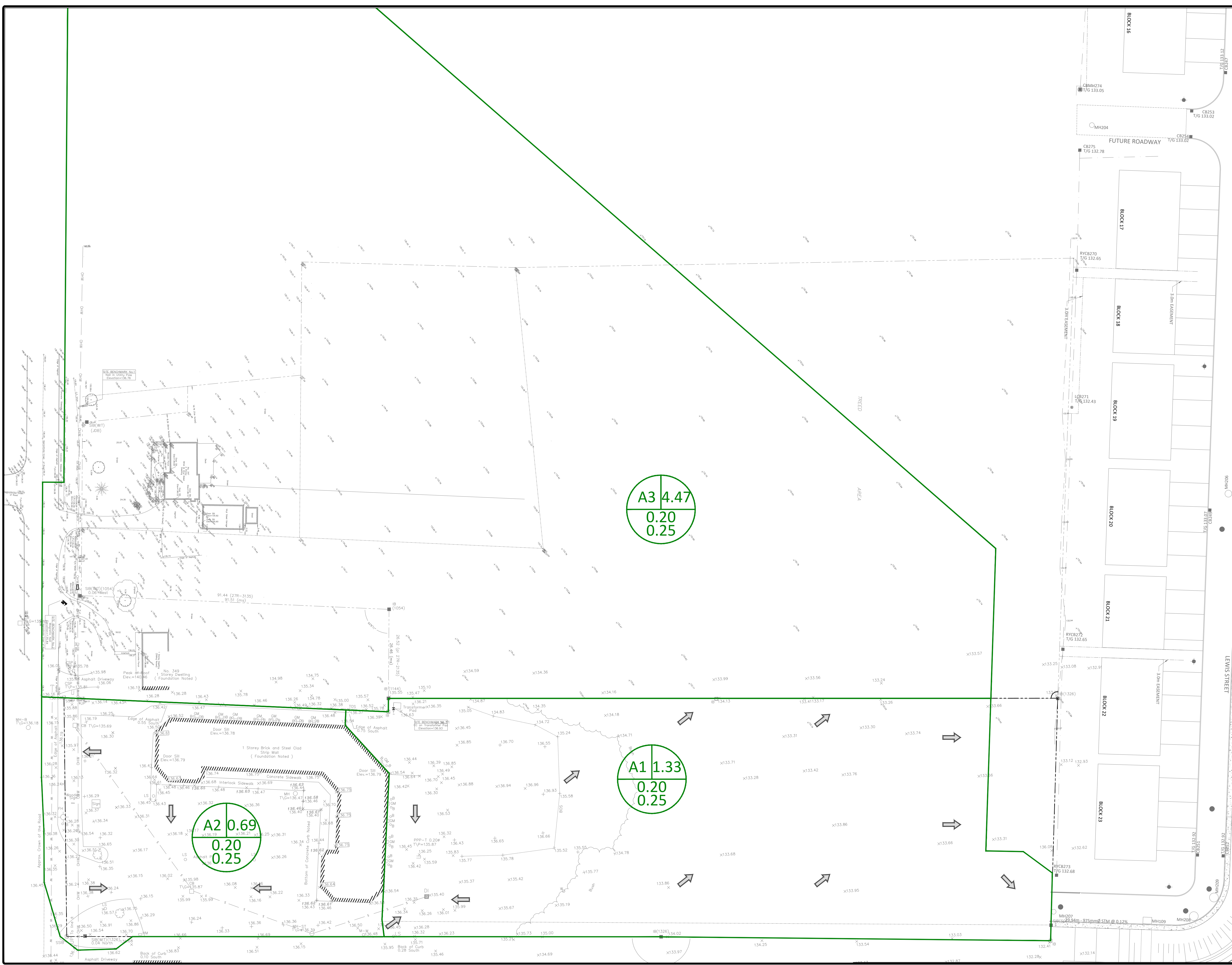
Project: **355 FRANKTOWN ROAD**
 CARLETON PLACE, ONTARIO

Drawing Title: **SANITARY DRAINAGE PLAN**

Scale: 1:500	Project Number: CCO-22-0402
Drawn By: C.H.	Checked By: B.C.
Designed By: C.H.	Drawing Number: SAN

APPENDIX E
PRE-DEVELOPMENT DRAINAGE PLAN

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LEGEND

AREA NAME

AREA (Ha)

5yr & 100yr COEFFICIENT

PRE-DEVELOPMENT DRAINAGE DIRECTION

FOR REVIEW ONLY
NOT FOR CONSTRUCTION

No.	Revisions	Date
3	REVISED AS PER COMMENTS	MAR. 15, 2024
2	REVISED AS PER COMMENTS	SEPT. 1, 2023
1	ISSUED FOR REVIEW	JUNE 30, 2022

Check and verify all dimensions before proceeding with the work. Do not scale drawings.

SCALE 1 : 500

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Stamp:

Client:

Project:

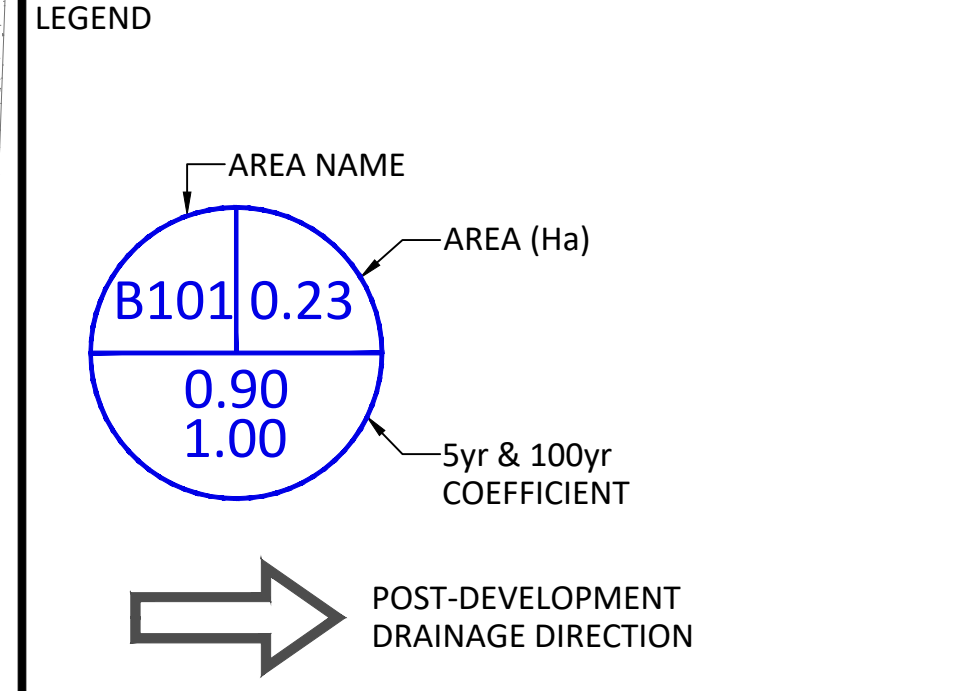
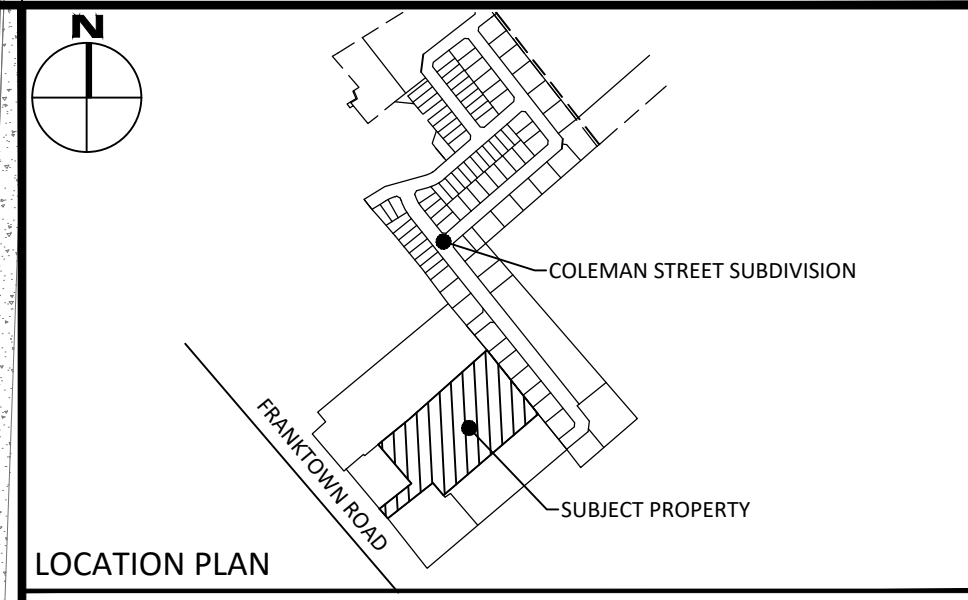
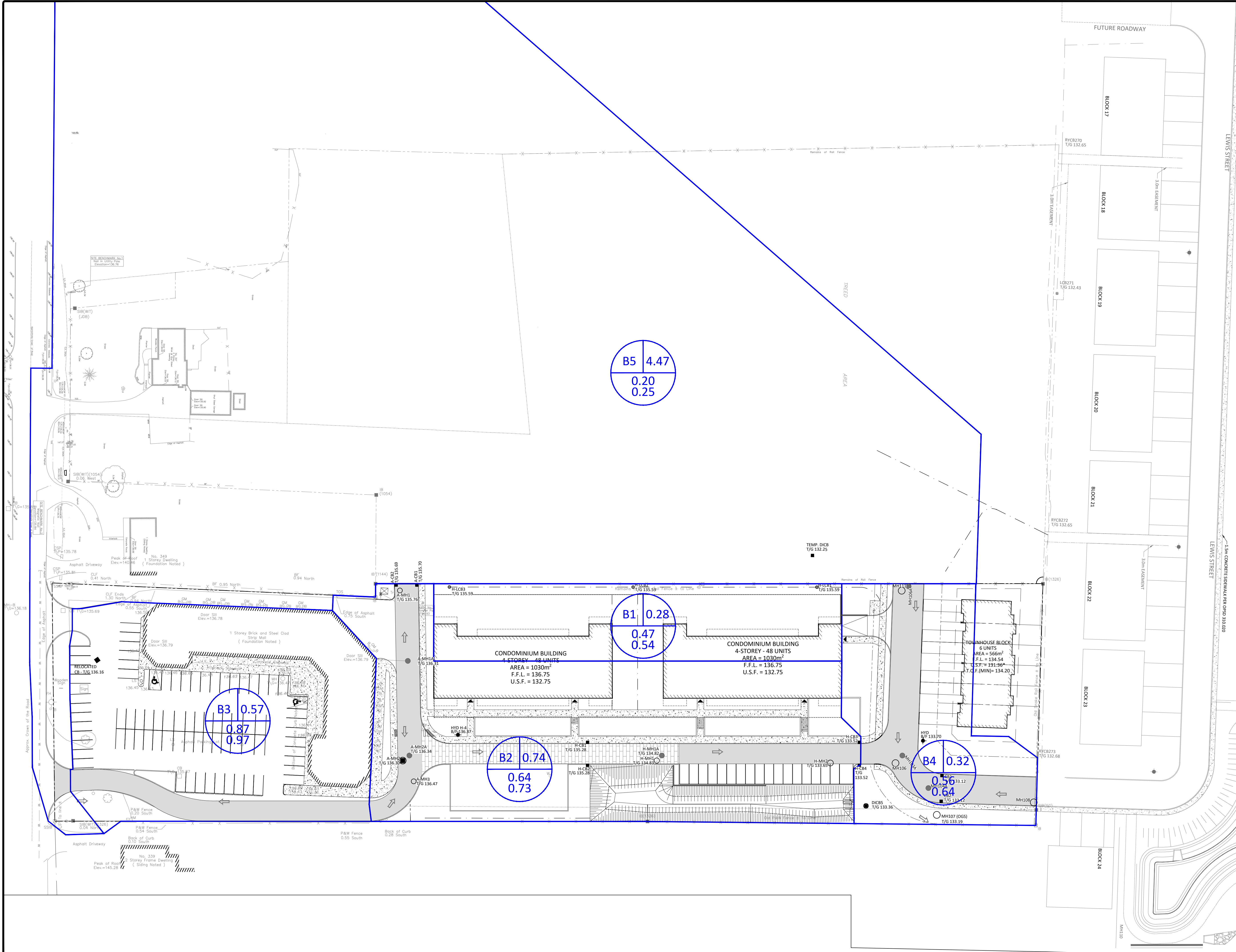
355 FRANKTOWN ROAD
CARLETON PLACE, ONTARIO

Drawing Title:

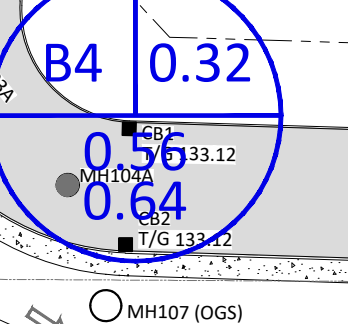
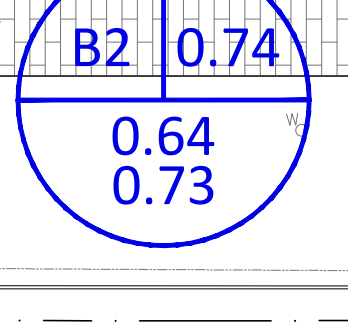
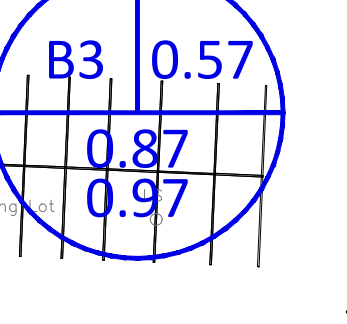
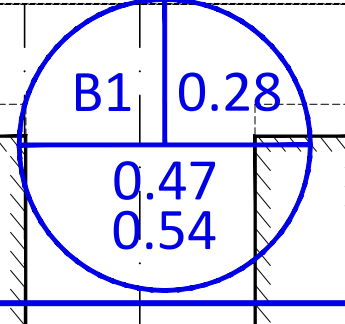
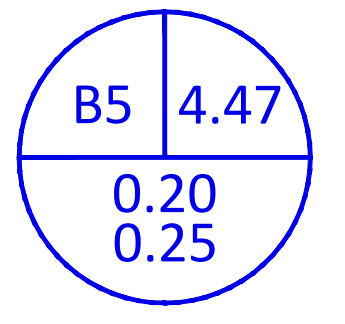
PRE-DEVELOPMENT DRAINAGE PLAN

Scale:	1:500	Project Number:	CCO-22-0402
Drawn By:	C.H.	Checked By:	B.C.
Designed By:	C.H.	Drawing Number:	PRE

APPENDIX F
POST-DEVELOPMENT DRAINAGE PLAN

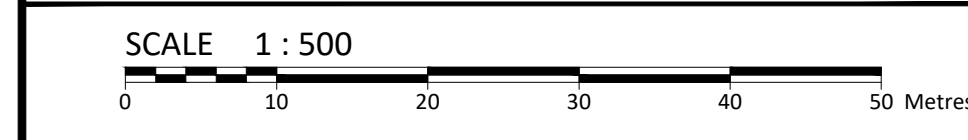


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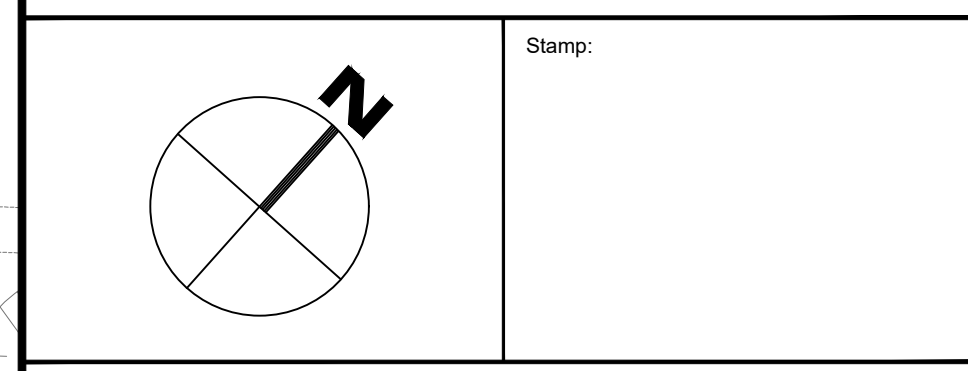


4	ISSUED FOR REVIEW	JUNE 13, 2024
3	REVISED AS PER COMMENTS	MAR. 15, 2024
2	REVISED AS PER COMMENTS	SEPT. 1, 2023
1	ISSUED FOR REVIEW	JULY 14, 2022
No.	Revisions	Date

Check and verify all dimensions before proceeding with the work. Do not scale drawings.



McINTOSH PERRY
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Client:

Project:

355 FRANKTOWN ROAD
 CARLETON PLACE, ONTARIO

Drawing Title:

POST-DEVELOPMENT DRAINAGE PLAN

Scale:	1:500	Project Number:	CCO-22-0402
Drawn By:	C.H.	Checked By:	B.C.
Designed By:	C.H.	Drawing Number:	POST

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APPENDIX G
STORMWATER MANAGEMENT CALCULATIONS

McINTOSH PERRY

COO-22-0402 - 355 Franktown Road

1 of 3

Tc (min)	Intensity (mm/hr)		
	5-Year	100-Year	
10	104.2	178.6	PRE-DEVELOPMENT
10	104.2	178.6	POST-DEVELOPMENT

C-Values	
Impervious	0.90
Gravel	0.60
Pervious	0.20

Pre-Development Runoff Coefficient

Drainage Area	Area (ha)	Impervious Area (m ²)	Gravel (m ²)	Pervious Area (m ²)	Average C (5-year)	Average C (100-year)
A1	1.33	0	0	13,310	0.20	0.25
A2	0.69	0	0	6,912	0.20	0.25
A3	4.47	0	0	44,743	0.20	0.25

Pre-Development Runoff Calculations

Drainage Area	Area (ha)	C 5-Year	C 100-Year	Tc (min)	Q (L/s)	
					5-Year	100-Year
A1	1.33	0.20	0.25	10	77.11	165.18
A2	0.69	0.20	0.25	10	40.04	85.78
A3	4.47	0.20	0.25	11	259.20	555.25
Total	6.49				376.35	806.21

Post-Development Runoff Coefficient

Drainage Area	Area (ha)	Impervious Area (m ²)	Gravel (m ²)	Pervious Area (m ²)	Average C (5-year)	Average C (100-year)
B1	0.28	1,064	0	1,733	0.47	0.54
B2	0.74	4,671	0	2,687	0.64	0.73
B3	0.57	5,444	0	208	0.87	0.97
B4	0.32	1,632	0	1,523	0.56	0.64
B5	4.47	0	0	44,743	0.20	0.25

North
South
Mall
Townhouse and Municipal Road
Offsite Area

Post-Development Runoff Calculations

Drainage Area	Area (ha)	C 5-Year	C 100-Year	Tc (min)	Q (L/s)	
					5-Year	100-Year
B1	0.28	0.47	0.54	10	37.77	74.32
B2	0.74	0.64	0.73	10	137.34	265.21
B3	0.57	0.87	0.97	10	143.13	272.82
B4	0.32	0.56	0.64	10	51.37	99.91
B5	4.47	0.20	0.25	10	259.20	555.25
Total	6.36				628.80	1267.51

Rear Swale / Building
Front Building and Road
Mall
Townhouse and Municipal Road
Offsite Area

Required Restricted Flow

Drainage Area	Area (ha)	C 5-Year	C 100-Year	Tc (min)	Q (L/s)	
					5-Year	100-Year
A1	1.33	0.20	0.25	10	77.11	165.18
A2	0.69	0.20	0.25	10	40.04	85.78
Total	2.02				117.15	250.95

Post-Development Restricted Runoff Calculations

Drainage Area	Unrestricted Flow		Restricted Flow		Storage Required (m ³)		Storage Provided (m ³)	
	5-year	100-Year	5-Year	100-Year	5-Year	100-Year	5-Year	100-Year
B1	37.77	74.32						
B2	137.34	265.21	55.77	135.10	195.9	331.7	199.9	353.7
B3	143.13	272.82						
B4	51.37	99.91	51.37	99.91	x	x	x	x
Total	369.60	712.25	107.13	235.01	195.94	331.70	199.94	353.71

McINTOSH PERRY

COO-22-0402 - 355 Franktown Road

2 of 3

Storage Requirements for Area B1, B2, B3

5-Year Storm Event

Tc (min)	I (mm/hr)	Runoff (L/s) B1, B2, B3	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	104.2	318.25	55.77	262.49	157.49
15	83.6	255.34	55.77	199.57	179.61
20	70.3	214.72	55.77	158.95	190.74
25	60.9	186.01	55.77	130.24	195.35
30	53.9	164.63	55.77	108.86	195.94
35	48.5	148.13	55.77	92.36	193.96
40	44.2	135.00	55.77	79.23	190.15
45	40.6	124.00	55.77	68.23	184.23
50	37.7	115.15	55.77	59.38	178.13
Maximum Storage Required 5-year =				195.9	m ³

100-Year Storm Event

Tc (min)	I (mm/hr)	Runoff (L/s) B1, B2, B3	Allowable Outflow (L/s)	Runoff to be Stored (L/s)	Storage Required (m ³)
10	178.6	612.49	135.10	477.38	286.43
15	142.9	490.06	135.10	354.95	319.46
20	120.0	411.52	135.10	276.42	331.70
25	103.8	355.97	135.10	220.86	331.30
30	91.9	315.16	135.10	180.06	324.10
35	82.6	283.27	135.10	148.16	311.14
40	75.1	257.55	135.10	122.44	293.86
45	69.1	236.97	135.10	101.87	275.04
50	64.0	219.48	135.10	84.38	253.13
Maximum Storage Required 100-year =				331.7	m ³

5-Year Storm Event Storage Summary

		Water Elev. (m) = 133.17	
Location	BOTTOM	Area (m ²)	Depth (m)
POND	132.40	439.1	0.77
		Volume (m ³) = 199.9	

ICD Location	INV. (out)	Head (m)
H-MH2	131.26	0.68

Storage Available (m ³) = 199.9
Storage Required (m ³) = 195.9

* Available Storage calculated from AutoCAD

100-Year Storm Event Storage Summary

		Water Elev. (m) = 133.47	
Location	BOTTOM	Area (m ²)	Depth (m)
POND	132.40	586.7	1.07
		Volume (m ³) = 353.7	

ICD Location	INV. (out)	Head (m)
H-MH2	131.26	0.98

Storage Available (m ³) = 353.7
Storage Required (m ³) = 331.7

* Available Storage calculated from AutoCAD

McINTOSH PERRY

COO-22-0402 - 355 Franktown Road

For Orifice Flow, C= 0.60
 For Weir Flow, C= 1.84

3 of 3

	Orifice 1	Orifice 2	Weir 1	Weir 2
invert elevation	132.40	X	133.40	X
center of crest elevation	132.49	X		X
orifice width / weir length	180 mm	X	2.00 m	X
weir height				X
orifice area (m ²)	0.025	X	x	X

Elevation Discharge Table - Storm Routing

Elevation	Orifice 1		Orifice 2		Weir 1		Weir 2		Total Q [L/s]
	H [m]	Q [m ³ /s]	H [m]	Q [m ³ /s]	H [m]	Q [m ³ /s]	H [m]	Q [m ³ /s]	
132.40	x	x	x	x	x	x	x	x	0.00
132.41	x	x	x	x	x	x	x	x	0.00
132.42	x	x	x	x	x	x	x	x	0.00
132.43	x	x	x	x	x	x	x	x	0.00
132.44	x	x	x	x	x	x	x	x	0.00
133.08	0.59	0.05	x	x	x	x	x	x	51.95
133.09	0.60	0.05	x	x	x	x	x	x	52.39
133.10	0.61	0.05	x	x	x	x	x	x	52.82
133.11	0.62	0.05	x	x	x	x	x	x	53.25
133.12	0.63	0.05	x	x	x	x	x	x	53.68
133.13	0.64	0.05	x	x	x	x	x	x	54.10
133.14	0.65	0.05	x	x	x	x	x	x	54.52
133.15	0.66	0.05	x	x	x	x	x	x	54.94
133.16	0.67	0.06	x	x	x	x	x	x	55.36
133.17	0.68	0.06	x	x	x	x	x	x	55.77
133.18	0.69	0.06	x	x	x	x	x	x	56.18
133.19	0.70	0.06	x	x	x	x	x	x	56.58
133.20	0.71	0.06	x	x	x	x	x	x	56.99
133.21	0.72	0.06	x	x	x	x	x	x	57.39
133.22	0.73	0.06	x	x	x	x	x	x	57.78
133.23	0.74	0.06	x	x	x	x	x	x	58.18
133.24	0.75	0.06	x	x	x	x	x	x	58.57
133.25	0.76	0.06	x	x	x	x	x	x	58.96
133.26	0.77	0.06	x	x	x	x	x	x	59.34
133.27	0.78	0.06	x	x	x	x	x	x	59.73
133.28	0.79	0.06	x	x	x	x	x	x	60.11
133.29	0.80	0.06	x	x	x	x	x	x	60.49
133.30	0.81	0.06	x	x	x	x	x	x	60.87
133.31	0.82	0.06	x	x	x	x	x	x	61.24
133.32	0.83	0.06	x	x	x	x	x	x	61.61
133.33	0.84	0.06	x	x	x	x	x	x	61.98
133.34	0.85	0.06	x	x	x	x	x	x	62.35
133.35	0.86	0.06	x	x	x	x	x	x	62.72
133.36	0.87	0.06	x	x	x	x	x	x	63.08
133.37	0.88	0.06	x	x	x	x	x	x	63.44
133.38	0.89	0.06	x	x	x	x	x	x	63.80
133.39	0.90	0.06	x	x	x	x	x	x	64.16
133.40	0.91	0.06	x	x	x	x	x	x	64.51
133.41	0.92	0.06	x	x	0.01	0.00	x	x	68.55
133.42	0.93	0.07	x	x	0.02	0.01	x	x	75.63
133.43	0.94	0.07	x	x	0.03	0.02	x	x	84.69
133.44	0.95	0.07	x	x	0.04	0.03	x	x	95.36
133.45	0.96	0.07	x	x	0.05	0.04	x	x	107.41
133.46	0.97	0.07	x	x	0.06	0.05	x	x	120.69
133.47	0.98	0.07	x	x	0.07	0.07	x	x	135.10
133.48	0.99	0.07	x	x	0.08	0.08	x	x	150.56
133.49	1.00	0.07	x	x	0.09	0.10	x	x	166.99
133.50	1.01	0.07	x	x	0.10	0.12	x	x	184.34
133.51	1.02	0.07	x	x	0.11	0.13	x	x	202.56
133.52	1.03	0.07	x	x	0.12	0.15	x	x	221.61
133.53	1.04	0.07	x	x	0.13	0.17	x	x	241.46
133.54	1.05	0.07	x	x	0.14	0.19	x	x	262.07
133.55	1.06	0.07	x	x	0.15	0.21	x	x	283.42
133.56	1.07	0.07	x	x	0.16	0.24	x	x	305.48
133.57	1.08	0.07	x	x	0.17	0.26	x	x	328.22
133.58	1.09	0.07	x	x	0.18	0.28	x	x	351.64
133.59	1.10	0.07	x	x	0.19	0.30	x	x	375.70
133.60	1.11	0.07	x	x	0.20	0.33	x	x	400.40

- Notes:
1. For Orifice Flow, User is to Input an Elevation Higher than Crown of Orifice.
 2. Orifice Equation: $Q = cA(2gh)^{1/2}$
 3. Weir Equation: $Q = CLH^{3/2}$
 4. These Computations Do Not Account for Submergence Effects Within the Pond Riser.
 5. H for orifice equations is depth of water above the centroid of the orifice.
 6. H for weir equations is depth of water above the weir crest.

STORM SEWER DESIGN SHEET

PROJECT: CCO-22-0402
 LOCATION: 355 Franktown
 CLIENT: Heafey Group



LOCATION				CONTRIBUTING AREA (ha)				RATIONAL DESIGN FLOW										SEWER DATA											
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28		
Phase	AREA ID	FROM MH	TO MH	C-VALUE	AREA	INDIV AC	CUMUL AC	INLET (min)	TIME IN PIPE	TOTAL (min)	i (5) (mm/hr)	i (10) (mm/hr)	i (100) (mm/hr)	5yr PEAK FLOW (L/s)	10yr PEAK FLOW (L/s)	100yr PEAK FLOW (L/s)	FIXED FLOW (L/s)	DESIGN FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)	PIPE SIZE (mm)		SLOPE (%)	VELOCITY (m/s)	AVAIL CAP (5yr)				
																					DIA	W	H			(L/s)	(%)		
		Temp. DICB	MH111	0.25	1.94	0.49	0.49	10.00	0.29	10.29	104.19	122.14	178.56	140.48		240.75			240.75	266.03	28.56	450			0.80	1.620	25.28	10%	
	B5	MH111	MH106	0.25	4.47	1.12	1.12	10.29	0.86	11.15	102.67	120.34	175.92	318.95		546.51			546.51	654.22	51.39	900			0.12	0.996	107.71	16%	
	B1	H-LCB1	H-LCB2	0.50	0.08	0.04	0.04	10.00	0.66	10.66	104.19	122.14	178.56	12.16					12.16	100.88	54.72	300			1.00	1.383	88.73	88%	
		H-LCB2	H-LCB3	0.42	0.12	0.05	0.09	10.66	0.66	11.32	100.83	118.18	172.75	26.24					26.24	100.88	54.72	300			1.00	1.383	74.64	74%	
		H-LCB3	A-CB1	0.50	0.07	0.04	0.13	11.32	0.11	11.43	97.71	114.50	167.34	35.49					35.49	100.88	9.29	300			1.00	1.383	65.39	65%	
	B2	A-CB1	A-CB2	0.78	0.09	0.07	0.20	11.43	0.08	11.52	97.20	113.90	166.45	55.29					55.29	100.88	7.00	300			1.00	1.383	45.59	45%	
	B2	A-CB2	A-MH1			0.00	0.20	11.52	0.03	11.54	96.82	113.45	165.80	55.08					55.08	100.88	2.24	300			1.00	1.383	45.81	45%	
	B2	A-MH1	A-MH2			0.00	0.20	11.54	0.74	12.29	96.69	113.31	165.59	55.01					55.01	129.34	50.66	375			0.50	1.134	74.33	57%	
	B3	A-MH3	A-MH2	0.87	0.57	0.49	0.49	10.00	0.07	10.07	104.19	122.14	178.56	143.13					143.13	182.91	6.98	375			1.00	1.604	39.79	22%	
	B2	A-MH2	H-MH1	0.77	0.18	0.14	0.83	12.29	0.89	13.17	93.49	109.54	160.05	216.79					216.79	317.25	75.63	525			0.50	1.420	100.45	32%	
	B2	H-MH1	H-MH2			0.00	0.83	13.17	0.61	13.78	89.96	105.38	153.95	208.61					208.61	317.25	51.72	525			0.50	1.420	108.64	34%	
	B2	H-MH2	SWM Outlet	0.68	0.27	0.18	1.02	13.78	0.10	13.88	87.71	102.74	150.07	248.31					248.31	845.52	11.18	750			0.53	1.854	597.21	71%	
		SWM Inlet	DICB5	0.38	0.19	0.07	1.09	13.88	0.07	13.95	87.35	102.31	149.44	265.06				55.77	55.77	87.74	7.05	250			2.00	1.731	31.97	36%	
		DICB5	MH107			0.00	1.09	13.95	0.11	14.06	87.11	102.03	149.03	264.33				55.77	55.77	905.89	20.23	600			2.00	3.104	850.12	94%	
	B4	MH106	MH107	0.56	0.32	0.18	1.29	11.15	0.31	11.47	98.47	115.40	168.66	48.55					546.51	595.06	809.89	19.71	975			0.12	1.051	214.83	27%
		MH107	MH108				1.29	14.06	0.45	14.51	86.73	101.58	148.36	42.76					602.28	645.04	809.89	28.56	975			0.12	1.051	164.85	20%
		MH108	MH109				1.29	14.51	0.47	14.99	85.17	99.75	145.67	41.99					602.28	644.27	809.89	29.94	975			0.12	1.051	165.62	20%
		MH109	MH110				1.29	14.99	0.56	15.54	83.60	97.90	142.97	41.21					602.28	643.50	784.83	39.10	525	3x525		0.34	1.171	141.33	18%
		MH110	HEADWALL				1.29	15.54	0.32	15.87	81.84	95.83	139.93	40.35					602.28	642.63	784.83	22.62	525	3x525		0.34	1.171	142.20	18%

Definitions:
 Q = 2.78CIA, where:
 Q = Peak Flow in Litres per Second (L/s)
 A = Area in Hectares (ha)
 i = Rainfall intensity in millimeters per hour (mm/hr)
 [i = 998.071 / (TC+6.053)^0.814] 5 YEAR
 [i = 1174.184 / (TC+6.014)^0.816] 10 YEAR
 [i = 1735.688 / (TC+6.014)^0.820] 100 YEAR

Notes:
 1. Mannings coefficient (n) = 0.013

Designed:
 FV

Checked:
 RF

Project No.:
 CCO-22-0402

Revision

No.	Revision	Date
1.	ISSUED FOR REVIEW	2022-07-15
2.	REVISED AS PER COMMENTS	2023-08-25
3.	ISSUED FOR REVIEW	2024.03.13

Date:
 2022-07-15

Sheet No.:
 1 of 1



Legend

- Baseplan
- STM Servicing Areas
- STM PIPES
- STM MHs
-
- Junctions**
- Visible
- Flooding Nodes
-
- ▲ Outfalls
- Storages
- Conduits
- Orifices
- Weirs
- Subcatchments



100 m

PCSWMM Report

355 Franktown Road - PCSWMM Report
Model 22-31376-PCSWMMModel_V3.0.inp

March 14, 2024

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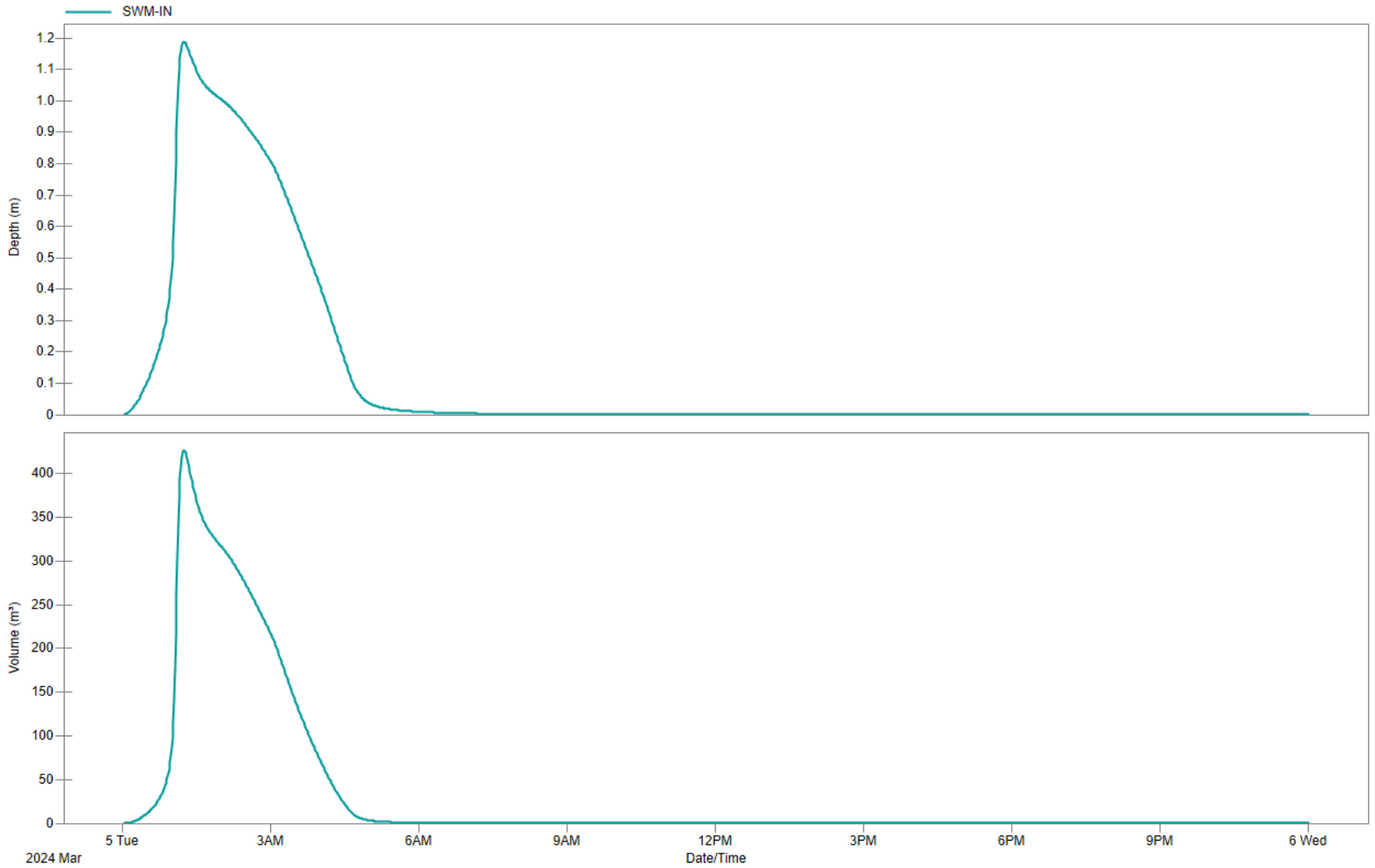


Figure 1: SWM Facility

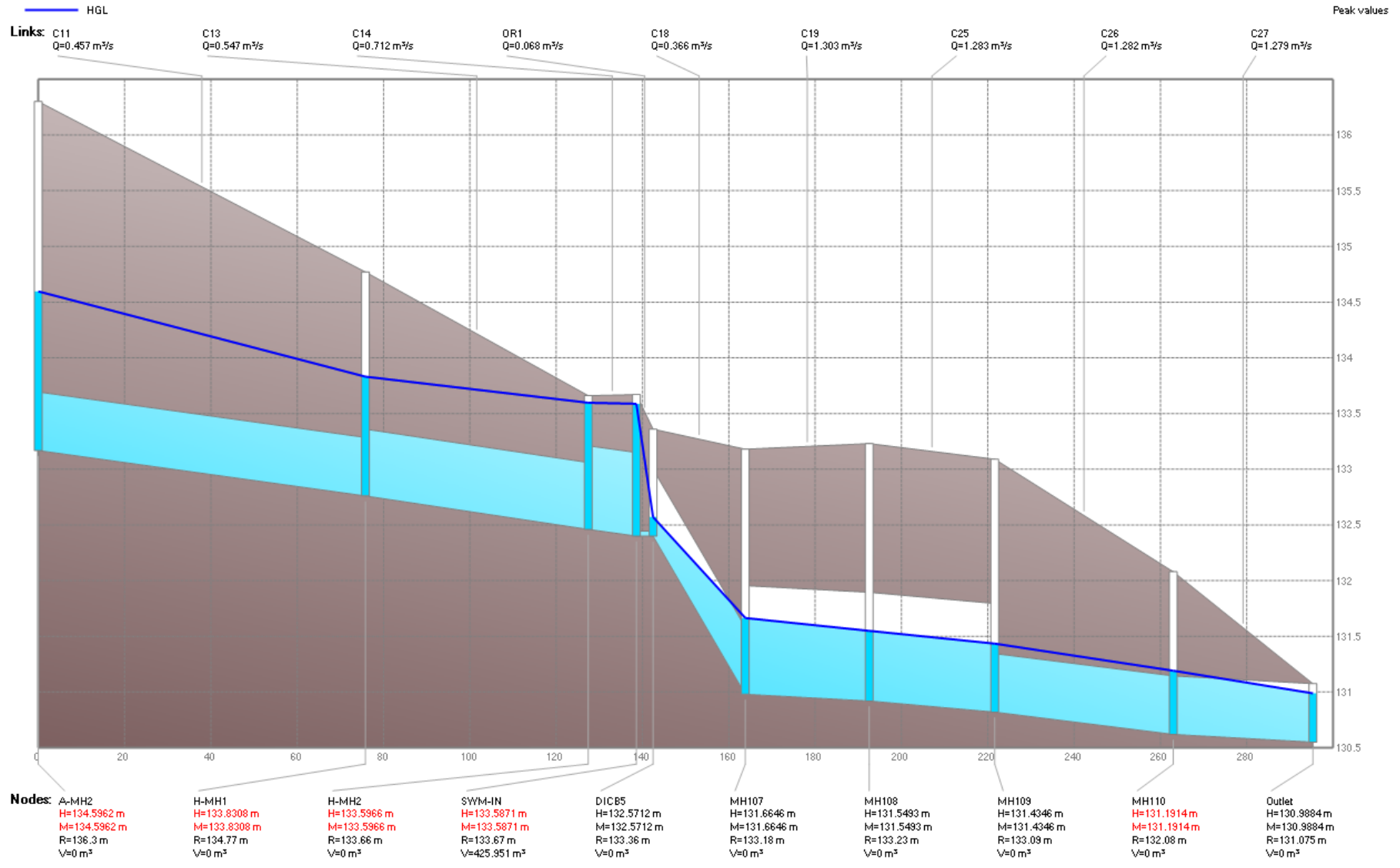


Figure 2: Stormsewer Profile

Table 1: Subcatchments

Name	X-Coordinate	Y-Coordinate	Rain Gage	Outlet	Area (ha)	Width (m)	Flow Length (m)	Slope (%)	Imperv. (%)	Peak Runoff (m ³ /s)
B1A	334097.944	4999159.014	Chicago_3H_100YR	H-LCB1	0.084	56	15	2.9	50	0.06
B1B	334064.503	4999129.269	Chicago_3H_100YR	H-LCB2	0.123	82	15	2	50	0.08
B1C	334032.732	4999100.894	Chicago_3H_100YR	H-LCB3	0.074	49.333	15	2.3	50	0.05
B2A	334019.289	4999078.383	Chicago_3H_100YR	A-CB1	0.095	63.333	15	2.8	80	0.06
B2B1	334039.871	4999050.956	Chicago_3H_100YR	A-MH2	0.04	26.667	15	2.4	80	0.03
B2B2	334057.009	4999084.764	Chicago_3H_100YR	H-CB1	0.137	91.333	15	1	80	0.09
B2C	334104.973	4999126.023	Chicago_3H_100YR	H-CB3	0.27	33.75	80	2.5	75	0.18
B2D	334094.834	4999086.742	Chicago_3H_100YR	SWM-IN	0.194	43.111	45	3.2	5	0.1
B3	334147.748	4999167.457	Chicago_3H_100YR	CB1	0.316	210.667	15	3	75	0.16
B3A	333958.809	4999027.189	Chicago_3H_100YR	REL-CB	0.096	64	15	2.5	98	0.06
B3B	333995.214	4999027.127	Chicago_3H_100YR	CB	0.43	286.667	15	1.7	98	0.29
B3C	334031.241	4999028.924	Chicago_3H_100YR	MH-ST	0.039	26	15	2.2	90	0.03
EXT	333947.194	4999161.914	Chicago_3H_100YR	TEMP-DICB	4.468	127.657	350	2.5	10	0.91

Table 2: Junctions

Name	Invert Elev. (m)	Rim Elev. (m)	Max. HGL (m)	Depth (m)	Time Max. HGL (M/D/Y)	Max. Total Inflow (m ³ /s)	Contributing Area (ha)	Flooding
A-CB1	132.46	135.7	135.05	3.24	03/05/2024 01:05 AM	0.171	0.376	0
A-CB2	133.52	135.69	135	2.17	03/05/2024 01:05 AM	0.171	0.376	0
A-MH1	133.48	135.76	134.99	2.28	03/05/2024 01:05 AM	0.172	0.376	0
A-MH2	133.168	136.3	134.67	3.132	03/05/2024 01:02 AM	0.458	0.981	0
A-MH3	133.3	136.49	134.86	3.19	03/05/2024 01:02 AM	0.28	0.565	0
CB	134.37	135.87	136.51	1.5	03/05/2024 01:07 AM	0.333	0.526	0.102
CB1	131.67	133.12	132.46	1.45	03/05/2024 01:05 AM	0.16	0.316	0
CB2	131.56	133.12	131.9	1.56	03/05/2024 01:05 AM	0.16	0.316	0
DICB5	132.4	133.36	132.57	0.96	03/05/2024 01:14 AM	0.366	1.582	0
H-CB1	133.82	135.26	134.45	1.44	03/05/2024 01:04 AM	0.092	0.137	0
H-CB2	133.46	135.26	134.28	1.8	03/05/2024 01:04 AM	0.092	0.137	0
H-CB3	132.57	133.52	133.6	0.95	03/05/2024 01:14 AM	0.175	0.27	0.041
H-CB4	132.54	133.51	133.6	0.97	03/05/2024 01:15 AM	0.174	0.27	0.026
H-LCB1	133.707	135.15	135.25	1.443	03/05/2024 01:09 AM	0.083	0.084	0.057
H-LCB2	133.137	135.59	135.25	2.453	03/05/2024 01:05 AM	0.099	0.207	0.025
H-LCB3	132.55	135.59	135.08	3.04	03/05/2024 01:05 AM	0.126	0.281	0
H-MH1	132.76	134.77	134.18	2.01	03/05/2024 01:03 AM	0.549	1.118	0
H-MH2	132.46	133.66	133.6	1.2	03/05/2024 01:15 AM	0.724	1.388	0
MH106	131.044	133.34	131.7	2.296	03/05/2024 01:12 AM	0.905	4.468	0
MH107	130.98	133.18	131.67	2.2	03/05/2024 01:12 AM	1.288	6.366	0
MH108	130.92	133.23	131.56	2.31	03/05/2024 01:12 AM	1.336	6.366	0
MH109	130.82	133.09	131.43	2.27	03/05/2024 01:12 AM	1.312	6.366	0
MH110	130.62	132.08	131.19	1.46	03/05/2024 01:13 AM	1.282	6.366	0
MH-111	131.162	133.92	131.85	2.758	03/05/2024 01:11 AM	0.902	4.468	0
MH-ST	134.89	136.39	135.68	1.5	03/05/2024 01:06 AM	0.28	0.565	0
REL-CB	134.66	136.16	136.55	1.5	03/05/2024 01:07 AM	0.075	0.096	0.085
TEMP-DICB	131.76	134.56	134.69	2.8	03/05/2024 01:08 AM	0.908	4.468	0.019

Table 3: Storages

Name	Rim Elev. (m)	Depth (m)	Initial Depth (m)	Storage Curve	Max. HGL (m)	Max. Volume (1000 m ³)	Max. Percent Full (%)	Max. Outflow (m ³ /s)
SWM-IN	133.67	1.27	0	TABULAR	133.59	0.426	88.5	0.366

Table 4: Conduits

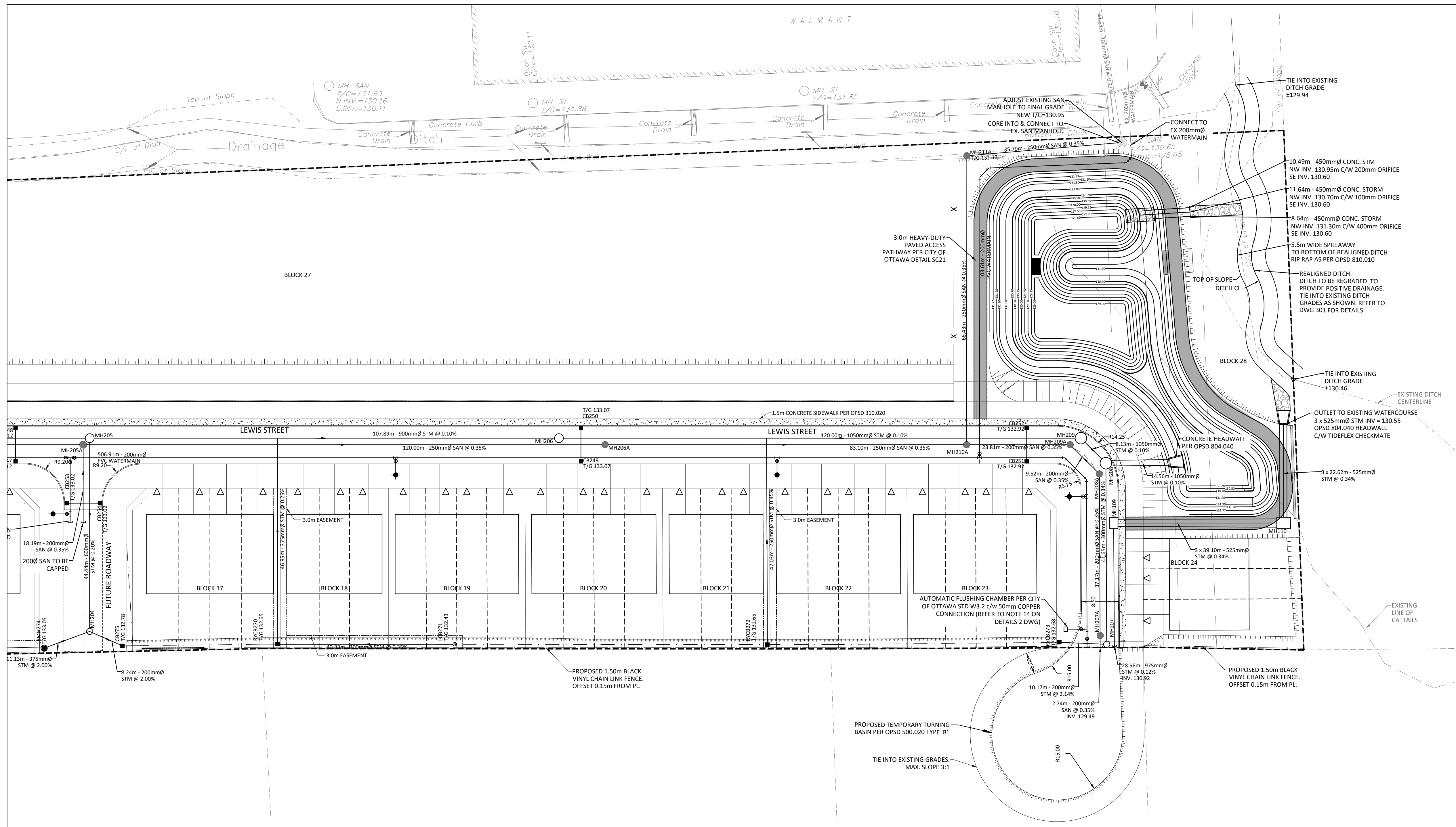
Name	Inlet Node	Outlet Node	Length (m)	Inlet Elev. (m)	Outlet Elev. (m)	Geom1 (m)	Barrels	Slope (m/m)	Max. Flow (m ³ /s)	Max. Velocity (m/s)	Inlet Structure
C1	REL-CB	CB	42.211	134.66	134.37	0.375	1	0.00687	0.071	0.64	
C10	A-MH3	A-MH2	7.036	133.3	133.168	0.375	1	0.01876	0.28	2.53	
C11	A-MH2	H-MH1	75.785	133.168	132.76	0.525	1	0.00538	0.458	2.11	
C12	H-CB2	H-MH1	20.712	133.46	132.76	0.25	1	0.03382	0.092	1.87	
C13	H-MH1	H-MH2	51.642	132.76	132.46	0.6	1	0.00581	0.549	1.94	
C14	H-MH2	SWM-IN	11.145	132.46	132.4	0.75	1	0.00538	0.725	1.64	
C15	H-CB1	H-CB2	6.863	133.82	133.46	0.25	1	0.05253	0.092	2.45	
C16	H-CB3	H-CB4	6.737	132.57	132.54	0.6	1	0.00445	0.174	0.7	
C17	H-CB4	H-MH2	8.687	132.54	132.46	0.6	1	0.00921	0.178	0.63	
C18	DICB5	MH107	21.386	132.4	130.98	0.6	1	0.06655	0.366	1.91	
C19	MH107	MH108	28.717	130.98	130.92	0.975	1	0.00209	1.336	2.5	
C2	CB	MH-ST	37.878	134.37	134.89	0.375	1	-0.01373	0.259	2.49	
C20	TEMP-DICB	MH-111	28.441	131.76	131.162	0.45	1	0.02103	0.902	5.67	
C21	MH-111	MH106	51.392	131.162	131.044	0.9	1	0.0023	0.905	1.85	
C22	MH106	MH107	19.652	131.044	130.98	0.975	1	0.00326	0.914	1.83	
C23	CB1	CB2	7.667	131.67	131.56	0.25	1	0.01435	0.16	3.25	
C24	CB2	MH107	4.209	131.56	130.98	0.25	1	0.13913	0.16	3.25	
C25	MH108	MH109	29.071	130.92	130.82	0.975	1	0.00344	1.312	2.88	
C26	MH109	MH110	41.366	130.82	130.62	0.525	3	0.00483	1.282	1.97	
C27	MH110	Outlet	32.374	130.62	130.55	0.525	3	0.00216	1.283	2.05	
C3	MH-ST	A-MH3	35.73	134.89	133.3	0.375	1	0.04454	0.28	2.58	
C4	H-LCB1	H-LCB2	54.623	133.707	133.137	0.3	1	0.01044	0.08	1.13	
C5	H-LCB2	H-LCB3	54.825	133.137	132.55	0.3	1	0.01071	0.099	1.41	
C6	H-LCB3	A-CB1	9.7	132.55	132.46	0.375	1	0.00928	0.126	1.14	
C7	A-CB1	A-CB2	6.183	132.46	133.52	0.375	1	-0.17401	0.171	1.55	
C8	A-CB2	A-MH1	1.983	133.52	133.48	0.375	1	0.02018	0.172	1.97	
C9	A-MH1	A-MH2	50.678	133.48	133.168	0.375	1	0.00616	0.173	1.56	

Table 5: Orifices

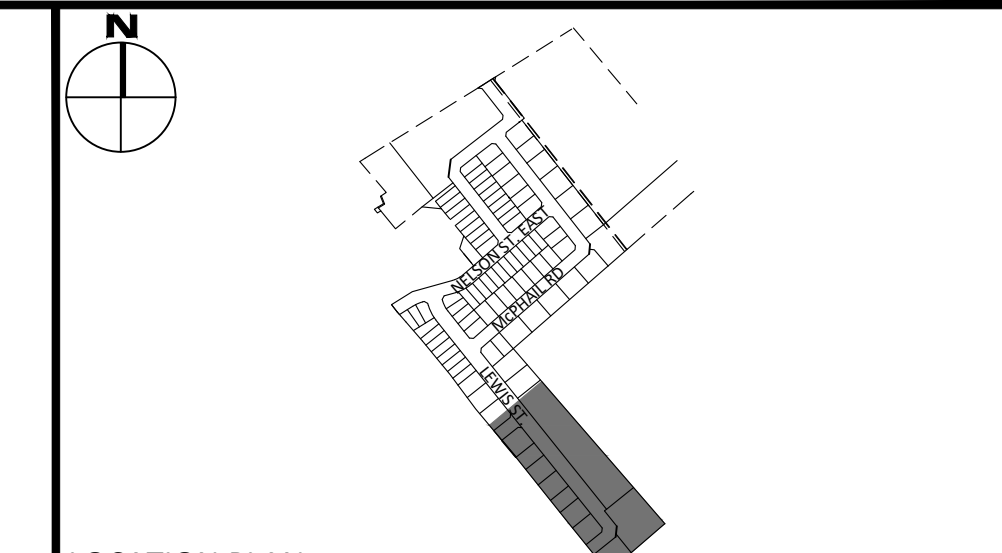
Name	Inlet Node	Outlet Node	Type	Cross-Section	Height (m)	Inlet Elev. (m)	Discharge Coeff.	Max. Flow (m ³ /s)	Capacity
OR1	SWM-IN	DICB5	BOTTOM	CIRCULAR	0.18	132.4	0.6	0.068	0

Table 6: Weirs

Name	Inlet Node	Outlet Node	Type	Height (m)	Length (m)	Side Slope (m/m)	Inlet Elev. (m)	Discharge Coeff. (m ³ /s)	End Coeff. (m ³ /s)	Max. Flow (m ³ /s)
W1	SWM-IN	DICB5	TRAPEZOIDAL	0.2	2	3	133.4	1.84	0	0.298



NOTE: ALL UNITS TO BE EQUIPPED WITH SUMP PUMP. REFER TO DRAWING "DETAILS 1".



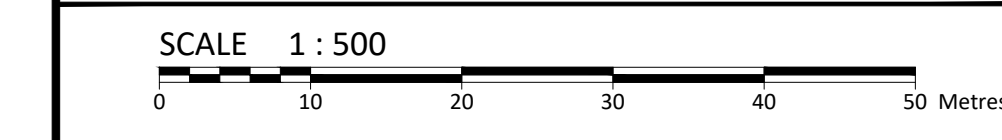
LEGEND

DC	CONCRETE MOUNTABLE CURB DEPRESSED	---	CENTRELINE OF SWALE SLOPING AT 3:1 (UNLESS SPECIFIED)
---	MOUNTABLE CURB	---	PROPOSED ELEVATION
---	EASEMENT	---	EXISTING ELEVATION
MH7	STORM MANHOLE	---	SWALE ELEVATION
CB6	DIG	---	TOP OF WALL ELEVATION
ECBA	T/CB4	---	BOTTOM OF WALL ELEVATION
MH7A	LANDSCAPE CATCHBASIN	---	FINISHED FLOOR
MH7A	SANITARY MANHOLE	---	TOP OF FOUNDATION
---	PERFORATED PIPE	---	UNDERSIDE OF FOOTING
---	WATER VALVE/CHAMBER	---	MINIMUM U.S.F.
---	SILT FENCE (AS PER OPSD 219.110)	---	ADDITIONAL RISERS
---	SEDIMENT CONTROL DEVICE	---	LIGHTWEIGHT FILL REQUIRED
---	STRAW BALE CHECK DAM (AS PER OPSD 219.180)	---	WALK OUT BASEMENT
---	TACTILE WALKING SURFACE INDICATOR (TWSI) PER OPSD 310.039	---	GRADE SPLIT UNIT
---	PRECAST BLOCK RETAINING WALL C/W PEDESTRAIN GAURD (BY OTHERS)	---	
---	APPROXIMATE EASEMENT LINE	---	

FOR REVIEW ONLY
NOT FOR CONSTRUCTION

No.	Revisions	Date
7	REVISED AS PER COMMENTS	FEB. 12, 2024
6	REVISED AS PER COMMENTS	NOV. 17, 2023
5	REVISED AS PER COMMENTS	MAY 31, 2023
4	REVISED AS PER COMMENTS	DEC. 5, 2022
3	REVISED AS PER COMMENTS	MAR. 21, 2022
2	REVISED LOT LINES	SEP. 23, 2021
1	ISSUED FOR REVIEW	AUG. 20, 2021

Check and verify all dimensions before proceeding with the work. Do not scale drawings.



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Client: **CAVANAGH DEVELOPMENTS**
9094 CAVANAGH RD, ASHTON, ON K0A 1B0

Project: **COLEMAN CENTRAL SUBDIVISION - PHASE 2**

Drawing Title: **GENERAL PLAN OF SERVICES**

Scale: 1:500	Project Number: CP-18-0360-01
Drawn By: C.H.	Checked By: B.C.
Designed By: C.H.	Drawing Number: 101

FILENAME: I:\Other\01 Project - Proposal\2018\01\CCO\CO-18-0360-01_Cavanagh_McGlobe_Subdivision_Ph_2_Cancom_Plan\CH112_Drawing\Production\CO-18-0360-01_DRAWINGS.SET.dwg
 DATE SAVED: Monday, February 12, 2024 1:51:54 PM
 LAST SAVED BY: J. Hall
 LAST PLOTTED: Monday, February 12, 2024 1:51:54 PM